

Bald Head Island, N.C. Beach Monitoring Program

Monitoring Report No. 19 (May 2020 to May 2021)

Prepared for: Village of Bald Head <u>Island</u>

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EXECUTIVE SUMMARY

The most recent Wilmington Harbor Inner Ocean Bar maintenance dredging of Bald Head Shoal Channel Ranges 1 and 2, and the Smith Island Channel range was performed in the months of January - April 2021. Federal surveys show approximately 1.6 Mcy of sand during that operation were placed along South Beach pursuant to the terms of the Wilmington Harbor Sand Management Plan (WHSMP). Bald Head Island will likewise be the recipient of the next *future* beach disposal operations in accordance with the continued implementation of the present day WHSMP. The last sand placement project constructed by the Village was between 13 January 2019 and 22 March 2019. The borrow site for that project was Jay Bird Shoals. The final fill volume (in-place) was 1.1 Mcy which included the addition of a Post-Florence FEMA Claim for documented storm related losses from the *engineered beach* in September 2018 (Olsen 2018). The limits of that fill extended eastward only to Sta. 146+00.

As part of the assessment for the 2019 beach renourishment project constructed at South Beach by the Village, it was determined that numerous sand tube groins had reached the end of their effective life and that replacement was warranted. Permits allowed for both an extension of time beyond April 1st for *both removal and replacement* of all remaining thirteen (13) sand tube groins (and underlayments). The work was initiated on/about 13 February and Substantially Completed by 22 March 2019. A *Post-Construction Report*, formulated to document the 2018-2019 project, details all elements of work performed by both contractors (Olsen 2019).

By about 2013, the results of a comprehensive annual beach monitoring program initiated in 2000 by the Village of Bald Head Island yielded the conclusion that sand placement alone could *not* successfully offset navigation channel impacts to the west end of South Beach which had been typically manifest in chronic rates of erosion and a consistent northerly post-fill recession of the shorefront. Accordingly, the Village was ultimately forced to "change the existing dynamic" by constructing a single terminal groin designed to complement the placement of future beach fills at the persistent South Beach erosional "hot spot". The project was permitted to be constructed in two phases – with Phase 2 being optional. Simplistically, the structure was designed to serve as a "template" for fill material placed eastward thereof on South Beach. The Phase 1 1,300 ft. long terminal groin (completed in Nov. 2015), was designed however as a "leaky" structure (*i.e.* semi-permeable) so as to provide for some level of continued sand transport to West Beach and portions of the Point (located both westward and northward of the groin stem). Through May 2021, terminal groin project performance – based upon post-construction monitoring – has been both as intended – and as predicted.

Between November 2000 and April 2021, Bald Head Island has received about 8.6 Mcy, mol of sand from the initial widening/deepening and five (5) subsequent maintenance dredging operations for the Wilmington Harbor Navigation Project entrance channel. Including the 2019 project, the Village has placed another 3.2Mcy of sand along the West Beach and South Beach shorelines. Accordingly, in the net Bald Head Island has experienced a total estimated sand placement volume of approximately 11.8 Mcy since 2000 at those two locations – with South Beach receiving 97% or more of the total.

Conversely, the gross volumetric sediment loss over a November 2000 to November 2020 (pre-disposal) monitoring timeframe is conservatively computed at 8.036 Mcy, or approximately 401,800 cy per year - on "average". This annualized "loss" addresses the continuous section of Bald Head Island shorefront extending from the marina entrance to the Cape Fear spit. The assignment of an average annual long-term rate of sand loss at Bald Head Island however, has *not* necessarily been a meaningful indicator of navigation project impact. Such an average rate is often temporally biased by periods of beach fill equilibration, groinfield "effectiveness due to reconstruction," recent storm events (such as Hurricanes Florence, Dorian and Isaias), the occurrence of episodic destabilization dredging events in close proximity to the island, as well as other physiographic phenomena temporally affecting annualized quantities of alongshore sediment transport - from Bald Head Island. In addition, the island's littoral system continues to adjust to the quasi-stabilizing effect of the terminal groin in existence only since 2015. Along South Beach per se, there has existed historically a "nodal point" some 7,000 ft. eastward of the terminal groin (approx. STA 116+00). At or close to the nodal point, the directionality of net littoral transport on an annual basis changes from West (toward the groin) to East (toward Cape Fear). *Note* – depending on wave climatology, the condition and exposure of the sand tube groinfield, as well as other factors, the effective location of the nodal point can vary slightly along South Beach from year to year. Currently, within the 22,755 shoreline influenced by sand episodically placed since 2000, up to 3.75 Mcy remain in the littoral system (measured above elevation -16 ft. NGVD 29). This includes the 1.6 Mcy beach disposal project recently completed in early April, 2021 by the Wilmington District, USACE.

Although not directly impacted by long-term navigation channel improvements and maintenance of the Cape Fear River entrance, the Village Council elected to initiate monitoring of the East Beach shorefront at Bald Head Island beginning in November 2008. Since that time, it is documented that East Beach can undergo strong seasonal variations of beach width and profile volume to a large degree dependent upon storm frequency and intensity, as well as the ever-changing configuration of the Cape Fear spit. For example, the most recent May 2021 survey data show a negligible net shoreline volumetric change of approximately 400 cy (above elevation -16 ft NGVD). throughout the 6,000 ft East Beach shoreline lying northward of Cape Fear over the last 12 months. In the prior year, it had eroded by almost 89,000 cy – due to

storm impacts. Between November 2008 and May 2021, the total change had been +350,100 cy. Most of the volume increase had been associated with recent post-storm accretion of the Cape Fear spit shoreline facing Onslow Bay.

Typically, periods of episodic accretional configurations of the Cape Fear spit deemed beneficial to East Beach have corresponded to a high rate of erosion and duneline recession along the easternmost section of South Beach – directly seaward and westward of the Shoals Club facility. For example, between 2000 and 2020, the average MHWL erosion rate at that general location has been over -20 ft/yr – due to sand losses either directly or indirectly associated with the configuration of the Cape Fear spit formation. The most recent (2021) federal disposal project placed fill within 2,000 ft. mol. of the Shoals Club and Cape Fear. This sand source may serve to reduce the most recent erosional cycle evident at the easternmost of South Beach.

In 2021, the Village performed monitoring of the Jay Bird Shoals borrow site utilized to construct the non-federal 1.85 Mcy beach fill sponsored by the Village in 2009/10 and the 1.10 Mcy beach constructed in 2018/19. During the Year 11 monitoring period (May 2020 to May 2021), the second year following the 2018/19 project excavation, the entire permitted borrow site gained 77,300 cy (inclusive of the exclusion and buffer zones). As of May 2021, there is approximately 1.23 Mcy of material located within the permitted borrow site limits above the permitted cut elevation (-22 ft-NGVD). Much of that material is *not* however practically available for dredging at this time.

After an extension of the two marina entrance channel jetties in 2015, temporarily reduced shoaling within the navigation channel resulted in a corresponding reduced volume of disposal sand being place along the Row Boat Row shoreline. Although the Village had planned to continue to proactively bypass sand from the south jetty fillet (located at the distal end of West Beach) to the Row Boat Row shorefront, it became clear that the existing four (4) low level timber groins were not capable of providing an acceptable level of shoreline stabilization at that location.

Hence, near the end of the 2017 monitoring period, the Village initiated construction of two (2) shore parallel detached rock breakwaters located north of the marina entrance seaward of the Row Boat Row shoreline. The placement of breakwaters between existing groins northward of the marina entrance was intended to combine the attributes of each of the two types of stabilization structure so as to reduce the rate of sediment transport from the eroding shoreline caused principally by ferry/barge generated waves. The subject expanded shore stabilization project (detached breakwaters *and* existing groinfield) was designed to have a sand fill prior to construction. The source of the fill was the exiting Bald Head Creek borrow area. A previously permitted Bald Head Creek borrow area was dredged in early 2017 by Marcol Dredging. Some 26,000 cy were placed at Row Boat Row prior to breakwater

implementation. Since that time multiple channel maintenance/sand bypass operations have occurred – most with increasing volumes dredged. Typically, dredging is required twice a year on average. This is primarily due to an increasing northerly rate of sediment transport along West Beach caused by a continuing reconfiguration of the Point. As a result, the Village has been forced to perform an increased frequency of bypassing of sand farther northward of the stabilizing influence of the breakwaters. This required a 2020 modification of the permits associated with the limits of allowable beach disposal seaward of Row Boat Row.

In the spring of 2019, the Village resubmitted permit applications accompanied by indepth geotechnical studies and environmental analyses intended to develop a long term (and large scale) supplementary borrow site located within Frying Pan Shoals. The purpose of such a borrow site would be to both ensure compliance with Permit conditions necessitating the maintenance of the updrift fillet associated with the 2015 terminal groin project and to provide an interim source of beach quality material sufficient to meet future South Beach renourishment requirements – when pursuant to the existing tenets of the Wilmington Harbor Sand Management Plan, beach quality channel maintenance material excavated would be placed at Oak Island. Permitting for this borrow site continues.

An important secondary precept of the spring of 2019 beach fill project constructed by the Village was to allow for the replacement of a sand tube groinfield which had become damaged over time. During the spring 2021 federal channel maintenance project, the groin field in its entirety was again covered by beach fill. That disposal project completed in early April 2021 placed 1.61 Mcy of sand between Sta.60+00 and Sta.121+00, mol. on South Bend.

The original Permits for construction of the terminal groin at Bald Head Island stipulated that if the permittee elected to dredge more than 250,000 cy from the Jay Bird Shoals borrow site after 2015, limited monitoring of the eastern end of Oak Island must be performed. Accordingly, in November 2018, the Village initiated the requisite monitoring at Oak Island (Caswell Beach). The first report of findings for Oak Island followed a November 2019 monitoring survey. A second year monitoring report was issued in December 2020. In early 2021 it was formally agreed by DCM and the USACOE that based upon the results of the Year 2 report, the *Village's responsibility for continued monitoring of Oak Island has terminated*.

In 2019, the Port of Wilmington, NC both sponsored and formulated a Section 203 Report which proposes a plan to deepen and widen (in places), the Federal navigation project, which extends from the Atlantic Ocean up the Cape Fear River to the Port of Wilmington. The Village of Bald Head Island formally submitted several series of comments to-the-record which addressed deficiencies in the project analyses and which requested clarification to impacts addressed, or unaddressed by the consultant prepared report.

BALD HEAD ISLAND, N.C. Beach Monitoring Program Report No. 19 (May 2020 – May 2021)

TABLE OF CONTENTS

1.0		INTRODUCTION								
		Overview								
		hysical Setting								
	1.3 Monitoring Period Storm Effects (May 2020 to May 2021)									
		Vilmington Harbor Federal Navigation Channel & Sand Management Plan								
	1.5 H	listorical Erosion Control Activities (1991 to 2021)	. 13							
	1	.5.1 Channel Maintenance Beach Disposal & Beach Restorations - Chronology	13							
	1	.5.2 Erosion Control Structures (1996 to 2021) - Chronology								
2.0	PHY	SICAL MONTORING PROGRAM	. 15							
	2.1	Monitoring Baseline & Beach Profiles	. 15							
	2.2	Bald Head Creek Borrow Site Surveys	. 18							
	2.3	Jay Bid Shoal Borrow Site Surveys								
	2.4	Orthorectified Aerial Photography								
3.0	MONITORING (SURVEY) RESULTS									
	3.1	Methodology	. 21							
	3.2	Year 20: Monitoring Program (May 2020 – Nov. 2020 – May 2021)	. 22							
	3.3	West Beach, "The Point" and South Beach: Discussion	. 22							
		3.3.1 Survey Period: May 2020 to November 2020	. 22							
		3.3.2 Survey Period: November 2020 to May 2021								
		3.3.3 Year 19 Monitoring Results: May 2020 to May 2021								
		3.3.4 Long-Term Beach Changes: November 2000 to May 2020								
		3.3.5 MHWL Shoreline Position								
	~ .	3.3.6 Chronology of "The Point"								
	3.4	East Beach Shoreline Conditions								
	3.5	Row Boat Row Shoreline Conditions	. 49							
4.0	JAY	BIRD SHOALS BORROW SITE MONITORING (SURVEY) RESULTS	. 51							
5.0	ONG	OING PLANNED OR PROPOSED ACTIVITIES	. 57							
	5.1	Development of a Frying Pan Shoals Borrow Site	. 57							
	5.2	Wilmington Harbor Navigation Project	. 57							
	5.3	Wilmington Harbor Deepening Project	. 58							

6.0	SUMMARY AND CONCLUSIONS	59
7.0	REFERENCES	63

LIST OF APPENDICES

APPENDIX A:	Plotted Beach Profiles (November 2000 to May 2021)
APPENDIX B:	Aerial Photography (May 2021)
APPENDIX C:	Aerial Photography (November 2020)
APPENDIX D:	Aerial Photography (May 2020)
APPENDIX E:	Photo Chronology of "The Point"
APPENDIX F:	November 2020 and May 2021 Monitoring Survey Drawings

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1.0 INTRODUCTION

1.1 Overview

This engineering report presents measured physical changes along the South Beach, West Beach, East Beach and Row Boat Row shorelines of Bald Head Island (BHI) based principally upon both historical and updated monitoring surveys performed annually on behalf of the Village of Bald Head Island (Village). It likewise addresses actions taken by the Village or others which have or could affect shoreline conditions. More, specifically, this report addresses:

- (1) An overview of Bald Head Island's physical setting including a discussion of the Federal Navigation Channel and the status of the implementation for Wilmington Harbor Sand Management Plan (WHSMP).
- (2) Recent volume and shoreline position changes measured between monitoring surveys of May 2020, November 2020 and May 2021 along the West Beach, "the Point" and South Beach shorelines, as well as *long-term changes* since November 2000. Updates of East Beach and the Cape Fear Spit conditions are likewise provided, as well as near term changes for the Row Boat Row shoreline which receives episodic sand placement associated with routine marina entrance channel sand bypass operations.
- (3) A discussion of the most recent 2020/21 Federal Beach Disposal Project constructed along the majority of S. Beach between January and April 2021.
- (4) A discussion of a recent Section 203 Harbor Deepening Project Investigation initiated by the Wilmington Harbor Port Authority and its potential implications to Bald Head Island.
- (5) The documented physical characteristics of H. Isaias in August 2020.

1.2 Physical Setting

Bald Head Island is located in Brunswick County, North Carolina at approximately 33°51' N, 78°00' W (**Figure 1.1**). It is roughly 25 miles south of the City of Wilmington and 32 miles east of the South Carolina/North Carolina state line. It is the southernmost of the coastal barrier islands which form the Smith Island complex at the mouth of the Cape Fear River. The southeastern tip of the island is Cape Fear (also referred to as Cape Fear Point) from which Frying Pan Shoals extend seaward over 20 miles to the southeast.

The island's east and south shorelines, "East Beach" and "South Beach", front the Atlantic shoreline. The west shoreline, or "West Beach", fronts the Cape Fear River. The north side of the island is bounded by the Bald Head Creek estuary, Middle Island and Bluff Island. The Cape Fear River entrance, over one mile in width, separates Bald Head Island from Oak Island (or Caswell Beach).

The astronomical tides in the vicinity of Bald Head Island are semi-diurnal and have average mean and spring ranges of approximately 4.3 ft and 5.0 ft, respectively. Tidal datums for Bald Head Island are listed in **Table 1.1** and the predicted astronomical tides during the May 2019 to May 2020 monitoring period are plotted as **Figure 1.2**.

Datum	Elevation (ft-NGVD29 ²)
Mean Higher High Water (MHHW)	+2.82
Mean High Water (MHW)	+2.51
NAVD 1988	+1.10
Mean Tide Level (MTL)	+0.35
NGVD 1929	0.00
Mean Low Water (MLW)	-1.81
Mean Lower Low Water (MLLW)	-1.98

Table 1.1: Tidal datums for Bald Head Island, North Carolina¹.

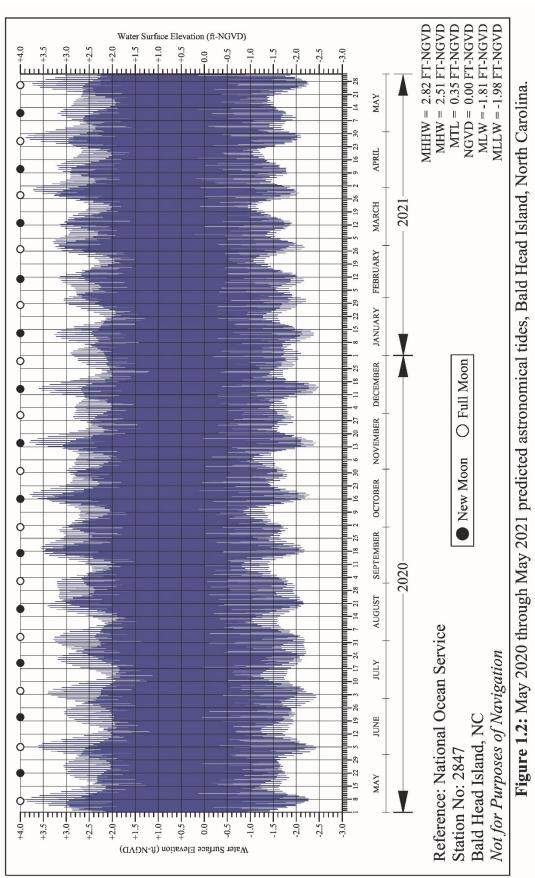
¹ Approximations based upon extrapolation from Southport, N.C.

² NGVD 1929: National Geodetic Vertical Datum of 1929 (1929 Mean Seas Level). Horizontal coordinates are referenced to the North Carolina State Plane Coordinate System, North American Datum of 1983.



Figure 1.1: Location of Bald Head Island, N.C. and Federal Navigation Channel.





1.3 Monitoring Period Storm Effects (May 2020 to May 2021)

In August 2020, both Oak and Bald Head Island experienced the effects of Hurricane Isaias. **Figure 1.3** plots the track of Hurricane Isaias as the storm approached the east coast of the United States. At the closest, the center of then Category 1 Hurricane Isaias passed about 33 miles west of the Cape Fear River, bringing sustained winds of 45 mph and gusts of 66 mph to the area³. On October 14th, 2020 Hurricane Isaias was declared a major disaster in 15 North Carolina counties, including Brunswick County, by the Federal Emergency Management Agency (FEMA DR-4568).

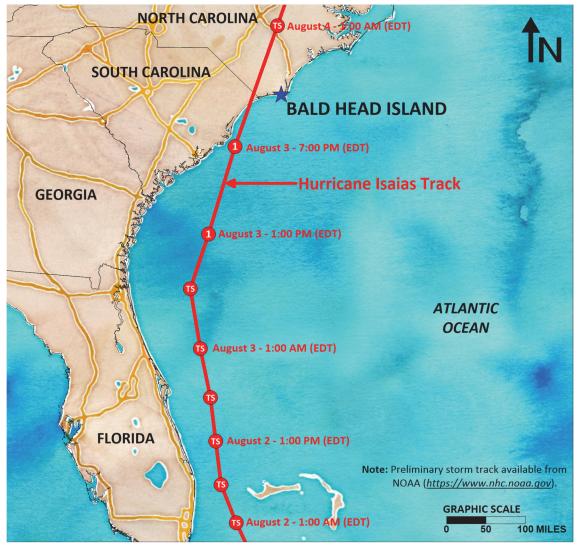


Figure 1.3: Hurricane Isaias (2020) track as the storm approached the coast of the United States. Track retrieved from the National Hurricane Center (NHC 2020).

³ Measured at the Cape Fear Regional Jet Port (KSUT).

Hurricane Isaias formed as a large tropical wave off the west coast of Africa on July 23, 2020 (NOAA 2020). The storm moved west and northwest across the Atlantic Ocean, reaching tropical storm strength on July 30, become the ninth named storm of the 2020 Atlantic hurricane season. Early on August 1, Isaias strengthened to a Category 1 hurricane just before making landfall on North Andros, Bahamas. As the storm crossed North Andros, it weakened to a tropical storm and made a northerly turn, paralleling the east coast of Florida and Georgia. As the storm approached the North and South Carolina coast, it strengthened back to Category 1 strength and made landfall near Ocean Isle Beach, North Carolina at approximately 10 pm on August 3rd (local time) with maximum sustained winds of 85 mph. At landfall, the center of the storm was located approximately 34 miles southeast of Oak Island. After landfall the storm moved up the east coast of the united stated before dissipating over Quebec, Canada on August 6th.

Figure 1.4 plots the track of Hurricane Isaias in the vicinity of Brunswick County. The figure also displays the location of NOAA Buoy 41108, located roughly 9 miles south of Bald Head Island in approximately 42 feet of water. The center of Hurricane Isaias passed within about 38 miles of the buoy. The buoy was deployed in March 1988 and has been collecting data nearly continuously for 33+ years except for an approximate 5 year period between April 1992 and May 1997. The data collected by the buoy includes significant wave height (average of the highest one-third of all waves in a 20-minute sampling period), wave period, wave direction, wind speed and other standard meteorological data.

Additionally, **Figure 1.4** shows the location of the Wrightsville Beach, NC tide station (NOS 8658163), owned and maintained by NOAA's National Ocean Service (NOS) Water Level Observation Network. The tide station is located at the offshore end of the Johnny Mercers Fishing Pier, approximately 28 miles northeast of the monitored area. In addition to water levels, the station collects readings of wind speed, gust speed, atmospheric pressure, and other meteorological data. The center of Hurricane Isaias passed within roughly 35 miles of the station.

Figure 1.5 depicts the time series measured at the buoy during the passage of Hurricane Isaias. While the buoy is located offshore in unprotected waters, the wave conditions measured can generally be related to nearshore conditions along the monitored area. Additionally, some fraction of the waves measured at the buoy are directed offshore. All wave statistics mentioned in this section will be from measurements at Buoy 41108.

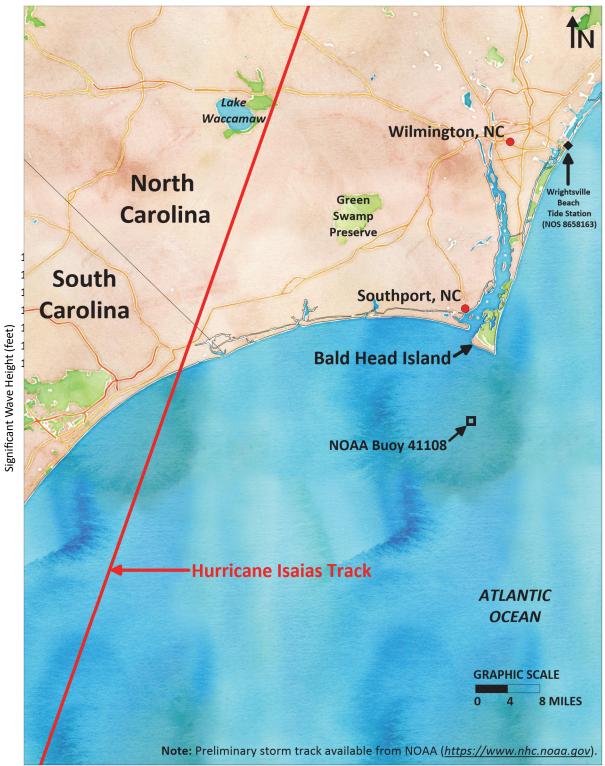


Figure 1.4: Track of Hurricane Isaias in the vicinity of Bald Head Island showing the location of NOAA Buoy 41108 and the Wrightsville Beach tide station (NOS 8658163). Track retrieved from the National Hurricane Center (NHC 2020).

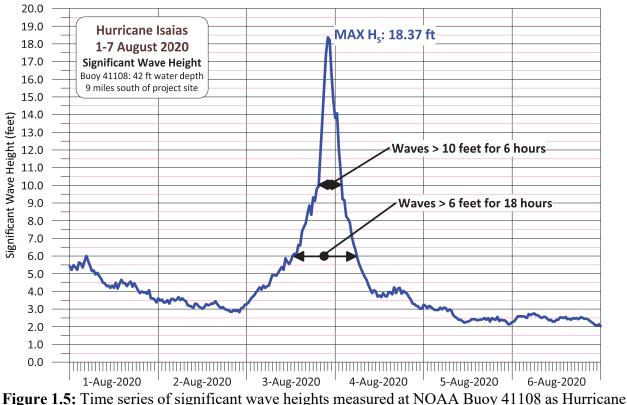


Figure 1.5: Time series of significant wave heights measured at NOAA Buoy 41108 as Hurricane Isaias moved by Bald Head Island, NC. The buoy is located approximately 9 miles south of Bald Head Island in 42 feet of water.

During Hurricane Isaias, significant wave heights peaked at 18.37 ft on the evening of August 3^{rd} , as the storm's center was approximately 40 miles east of the wave buoy. The dominant wave period peaked at about 12.5 seconds during this time. It is noted that these measurements reflect the significant wave height, or the average of the highest $1/3^{rd}$ of waves passing the buoy during a 20 minute sampling period. There are waves that passed during the storm that were larger in height – some waves may have been as much as twice as high. Large waves persisted along the coast for a significant duration of time with significant wave heights greater than 10 feet lasting for 6 hours and waves greater than 6 feet for 18 hours. For comparison, during Hurricane Dorian (September 2019), wave heights greater than 10 feet lasted for about 20 hours and waves greater than 6 feet for 46 hours.

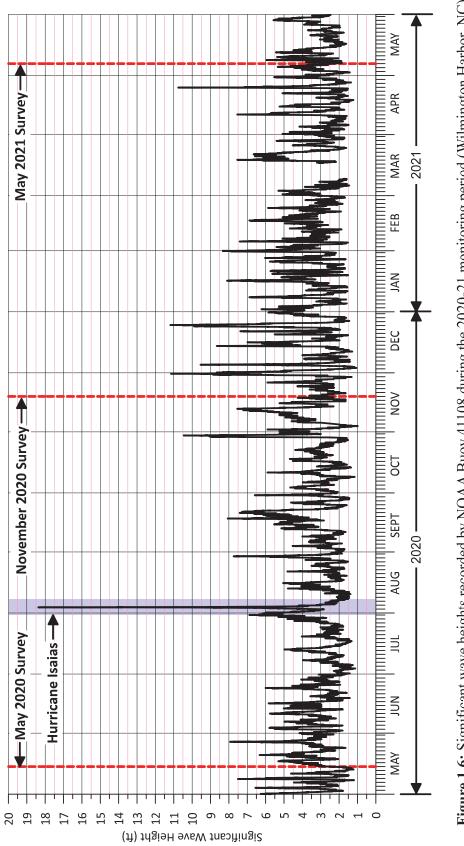
Figure 1.6 displays a time series of significant wave heights measured at NOAA Buoy 41108 from May 2020 through May 2021. The average significant wave height⁴ at NOAA Buoy 41108 during the intersurvey period (May 7, 2020 to May 7, 2021⁵) was 3.23 feet with a maximum wave height of 18.37 ft measured during Hurricane Isaias. The monitoring period's average value is slightly higher than the full record average significant wave height of 3.22 feet (March 1988 through May 2021).

During the monitoring period, roughly 4.6 percent of the recorded wave heights were above 6 feet, slightly less than the full record average of 5.7 percent. However, the occurrence of waves above 10 feet was the same as the full record average (0.2 percent).

Figure 1.7 displays the time series of water levels measured at the Wrightsville Beach, NC tide gage as Hurricane Isaias passed. The maximum water level recorded during Hurricane Dorian was approximately +5.14 ft-NGVD. This occurred approximately 40 minutes after the predicted high tide. The corresponding storm surge (i.e. level above predicted tides) at this time was +2.09 ft. The maximum storm surge of +4.45 ft occurred approximately 3 hours before low tide. At this time, the predicted water level was +0.13 ft-NGVD yielding a measured water level of +4.58 ft-NGVD.

⁴ These measurements reflect the significant wave height, or the average of the highest 1/3rd of waves passing the buoy during a 20 minute sampling period.

⁵ The May 2020 beach profile survey was completed May 7, 2020 and the May 2021 beach profile survey completed on May 7, 2021.





July 2021

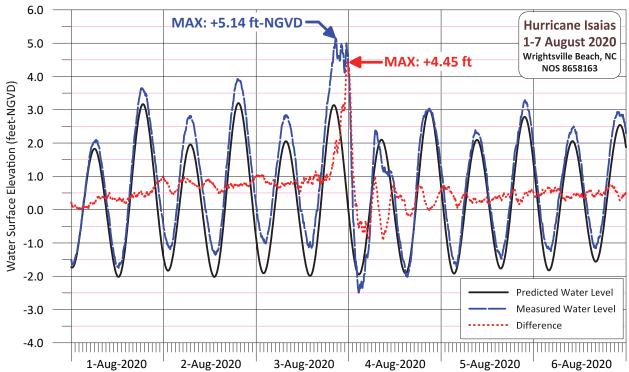


Figure 1.7: Time series of water levels measured at the Wrightsville Beach NOAA tide gage (NOS 8658163) located along the open Atlantic Coast, as Hurricane Isaias approached Brunswick County, NC.

1.4 Wilmington Harbor Federal Navigation Channel and Sand Management Plan

A detailed discussion of the history of the navigation channel and the Wilmington Harbor Sand Management Plan is provided in Monitoring Report No. 15 (Olsen 2017).

The Wilmington Harbor Federal Navigation Project extends up the Cape Fear River from a point seven statute miles seaward of the Bald Head Island Marina, upstream 30.4 miles to a location just north of the City of Wilmington, N.C. The Wilmington District, U.S. Army Corps of Engineers (USACE) is responsible for maintaining the project at its congressionally authorized depths and widths.

The Wilmington Harbor Sand Management Plan (USACOE 2000) was formulated as a specific action element of the deepening project for Wilmington Harbor. For the most part, the Plan was in direct response to the stated concerns of the Village of Bald Head Island regarding the historical harbor maintenance impacts and potential new impacts of the deepening project to both the regional sediment budget and Bald Head Island. The Plan's stated purpose was to reverse the practice of placing beach quality sand in the off-shore disposal area by calling for placement of this sand onto adjacent beaches. Over a theoretical six-year biennial maintenance cycle, the initial Wilmington Harbor Sand Management Plan (WHSMP) stipulated that approximately 1.0 Mcy of sand was to be placed on the beaches of Bald Head Island in years two and four (after initial construction) and on Oak Island/Caswell Beach during year six. The six-year disposal cycle was proposed for the life of the project but, accordingly to its terms, could be altered based upon documentation of impacts to adjacent beaches, changes in conditions and other relevant factors. The first six-year (3 maintenance event) cycle was completed in April 2009. In early 2011, the Wilmington District issued a draft report-of-findings both summarizing approximately 10-years of monitoring and readdressing the tenets of the original (2000) Sand Management Plan based upon their interpretation of monitoring results, related analyses and other salient factors or considerations. Subsequently the District solicited public comments from the two (2) principal stakeholders – the Village of Bald Head Island and Caswell Beach.

It has been OAI's continuing opinion that the division of sand between the two (2) abutting shorefronts of Oak Island and Bald Head Island should be based upon the cumulative quantities of sediment *lost* from each shoreline over the prior dredging cycle(s) as documented by survey, as well as identifiable impacts which exceed the November 2000 (pre-project) benchmark survey. Alternatively, it should be based upon the ratio of documented littoral transport rates for each island toward the Cape Fear River.

1.5 Historical Erosion Control Activities (1991 to 2021)

1.5.1 Channel Maintenance Beach Disposal and Beach Restorations – Chronology

Beach fill placement activities constructed at Bald Head Island since 1991 are summarized in **Table 1.2**. Not including disposal operations after 2018/19, a detailed discussion of the history of channel maintenance beach disposal and beach restoration activities is provided in Monitoring Report No. 15 (Olsen 2017).

Year	Volume	Sponsor	Location			
1991	$0.35\pm Mcy$	VBHI	(Sta. 24+00 to 138+00)			
1996	$0.65 \pm Mcy$	VBHI	(Sta. 24+00 to 142+00)			
1997	$0.45 \pm Mcy$	VBHI	(Sta. 24+00 to 128+00)			
2001	$1.849 \pm Mcy$	USACE*	South Beach (Sta. 41+60 to 205+50)			
2005	$1.217 \pm Mcy$	USACE*	South Beach (Sta. 46+00 to 126+00)			
2006	47,800 cy	VBHI	West Beach (Sta. 16+00 to 34+00)			
2007	$0.9785 \pm Mcy$	USACE*	South Beach (Sta. 46+00 to 174+00)			
2009/10	$1.850\pm Mcy$	VBHI	West Beach (Sta. 8+00 to 32+00) South Beach (Sta. 40+00 to 190+00)			
2012	137,990 cy	FEMA/VBHI	West Beach & Western South Beach			
2012	$1.566 \pm Mcy$		South Beach (Sta. 44+00 to 150+00)			
2013	92,500 cy	USACE*	West Beach (Sta. 8+00 to 27+00)			
2015	$1.33 \pm Mcy$	USACE*	South Beach (Sta. 41+50 to 154+00)			
2016/17	50,000 cy	VBHI	West Beach and Row Boat Row			
2018/19	1.1 Mcy	VBHI	South Beach (Sta. 49+00 to Sta. 146+00)			
2021	1.61 Mcy	USACE	South Beach (Sta. 60+00 to Sta. 212+00)			

 Table 1.2: Beach disposal or sand placement activities at Bald Head Island since 1991.

* Disposal pursuant to the WHSMP. Dredge volume estimate (pre-losses).

1.5.2 Erosion Control Structures (1996 to 2020) - Chronology

Erosion control structures constructed at Bald Head Island since 1996 are summarized in **Table 1.3**. A detailed discussion of the (pre-2021) history of erosion control structures is provided in Monitoring Report No. 15 (Olsen 2017).

Year	Location	Description
1994	Western South Beach	Sand bag revetment located along 645 feet of the back- beach berm
1996 (March)	Western South Beach	Sixteen (16) soft groins (geotube-type structures) were constructed of geotextile material and sand fill
2003/2004	Western South Beach	Rehabilitation of 1994 constructed sand bag revetment. Revetment lengthened by approximately 200 feet and base width increased to 40 ft and crest elevation raised to +12 ft-NGVD).
2005 (January to March)	Western South Beach	Replacement of 1996 constructed sand tube groin field. Minor changes in groin location were made in an effort to improve performance. Similarly, experimental "tapered" tubes were deployed in an attempt to better accommodate beach profile recession over time.
2009	Western South Beach	Complete rehabilitation of the sand tube groin field. Some adjustment of groin lengths, and the westward relocation of groin no. 16 were made in an attempt to refine the project design.
2011	Western South Beach	300 ft sand bag revetment was constructed on the downdrift (western side) of the last sand tube groin in order to protect several endangered residential structures.
2013	Western South Beach	In the spring of 2013, the westernmost five (5) sand tube groins were replaced in their entirety. This work was co- funded by FEMA as part of a post-Irene damage mitigation effort. The project P.W. was BHGJS04 in accordance with FEMA declaration 4019 DR NC.
2015	Western South Beach	In the spring of 2015, construction was initiated on a single 1,300 ft. long rock terminal groin designed to complement future placement of beach fill at South Beach. At that time, the westernmost three (3) geotube groins were removed in their entirety. A detailed description of the project is provided in Monitoring Report No. 15 (Olsen 2017).
2015	Bald Head Marina	The two marina entrance channel structures seaward of Row-Boat-Row originally constructed by Bald Head Island, Ltd., were modified through the addition of rock extensions.
2017	Row Boat Row	Two (2) detached breakwaters were constructed just north of the Marina Entrance. Construction details are provided in Monitoring Report No. 16 (Olsen, 2018).
2019	Western South Beach	All remaining 13 sand tube groins were removed and replaced coincident with the 2019 beach fill.

Table 1.3: History of erosion control structures at Bald Head Island since 1994.

2.0 PHYSICAL MONITORING PROGRAM

2.1 Monitoring Baseline & Beach Profiles

<u>MONITORING BASELINE</u> The present day Bald Head Island monitoring baseline extends roughly 31,400 ft from the northern end of Row Boat Row (Sta. -014+72), southward along West Beach, around "the Point", then eastward along South Beach to Cape Fear and finally northward along East Beach (Sta, 284+00). The individual profile stationing and coordinates are listed in **Table 2.1** and graphically depicted in **Figure 2.1**.

BEACH PROFILES In order to document and assess any potential adverse effects of the Wilmington Harbor Navigation Channel Navigation project to Bald Head Island, the Village Council initiated a comprehensive beach monitoring program which commenced in 1999. As part of the program onshore and offshore profiles are measured annually at seventy-nine (79) stations spaced approximately 400 ft apart along the roughly 31,400 ft of Bald Head Island's shorefront. **Table 2.2** summarizes the monitoring surveys conducted to date as part of the monitoring program. The primary focus of this monitoring report (No. 19) is beach profile and shoreline changes occurring over the latest set of surveys (May 2020 to May 2021).

Typically, survey transects extend across the upland berm or from the dune line seaward a distance of up to 3,000 ft. Depending upon the location of the survey profile, this distance corresponds to offshore waters depths of at least -40 ft relative to NGVD within the Cape Fear River Channel and -16 ft-NGVD along the Atlantic Ocean shorefront. In Chapter 5.0, these surveys are intra-compared in order to determine trends in the condition of the beaches of Bald Head Island. Plots of selected historical comparative beach profile data (through May 2021) are provided in **Appendix A**.

Prior to October 2003, fifty-five (55) stations were surveyed as part of the monitoring program. Five (5) additional intermediate stations were added at the Point, commencing with the October 2003 survey. These profile stations were added to more accurately capture the extreme changes that occur at the Point. Seven (7) profiles were added along East Beach (EB-01 to EB-07) beginning with the November 2008 survey. Beginning with the November 2015 survey five (5) profiles were added along Row Boat Row and four (4) were added at the Point, as part of the terminal groin monitoring requirement. Finally, in November 2016, three (3) additional profiles were added along West Beach.

Station Location Grid					Station I	Grid	
Station	Easting	Northing	Azi.		Easting	Northing	Azi.
(Monument)	(FT-NAD83)	(FT-NAD83)	(Deg.)	Station	(FT-NAD83)	(FT-NAD83)	(Deg.)
	Row Boat R		084+16 (B-22)	2,303,032.1	40,924.5	219	
-014+72 (RB-01)	2,304,277.9	49,117.4	302	088+23 (B-23)	2,303,372.1	40,705.0	214
-012+00 (RB-02)	2,304,068.6	48,776.5	302	092+15 (B-24)	2,303,714.1	40,513.9	209
-008+00 (RB-03)	2,303,937.2	48,538.1	302	097+10 (B-25)	2,304,146.1	40,272.5	206
-004+00 (RB-04)	2,303,728.0	48,197.2	302	102+08 (B-26)	2,304,592.1	40,057.6	204
-003+00 (RB-05)	2,303,518.7	47,856.3	302	106+00 (B-27)	2,304,960.4	39,915.3	201
	West Beac	h		110+00 (B-28)	2,305,333.5	39,771.1	201
000+00 (B-01)	2,303,309.3	47,515.5	302	114+00 (B-29)	2,305,708.5	39,626.3	202
004+00 (B-02)	2,303,100.4	47,174.4	301	118+00 (B-30)	2,306,080.6	39,482.5	202
008+00 (B-03)	2,302,891.5	46,833.3	301	122+00 (B-31)	2,306,451.7	39,339.2	201
010+00 (I-03)	2,302,788.1	46,662.0	301	126+00 (B-32)	2,306,824.0	39,195.3	200
012+00 (B-04)	2,302,682.5	46,492.2	301	130+00 (B-33)	2,307,196.5	39,051.4	200
014+00 (I-04)	2,302,578.8	46,321.1	301	134+00 (B-34)	2,307,569.6	38,907.3	200
016+00 (B-05)	2,302,473.6	46,151.1	301	138+00 (B-35)	2,307,943.9	38,767.8	200
018+00 (I-05)	2,302,369.5	45,980.3	301	142+00 (B-36)	2,308,320.5	38,633.0	200
020+00 (B-06)	2,302,264.7	45,810.0	301	146+00 (B-37)	2,308,697.1	38,498.2	200
024+00 (B-07)	2,302,055.2	45,468.8	302	150+00 (B-38)	2,309,073.8	38,363.4	200
	"the Point	**		154+00 (B-39)	2,309,452.4	38,228.0	201
028+00 (B-08)	2,301,845.1	45,126.6	303	158+00 (B-40)	2,309,818.8	38,074.6	202
032+00 (B-09)	2,301,566.1	44,843.7	301	162+00 (B-41)	2,310,179.1	37,895.6	203
034+00 (I-09)	2,301,394.4	44,742.0	301	166+00 (B-42)	2,310,539.0	37,716.9	204
036+00 (B-10)	2,301,220.2	44,647.1	299	170+00 (B-43)	2,310,903.5	37,552.0	204
038+00 (I-10)	2,301,043.1	44,550.6	296	174+00 (B-44)	2,311,267.9	37,387.2	204
039+60 (B-11)	2,300,902.6	44,473.9	291	178+00 (B-45)	2,311,632.4	37,222.3	204
041+50 (I-11)	2,300,765.0	44,365.0	287	182+00 (B-46)	2,311,996.9	37,057.4	204
043+47 (B-12)	2,300,757.5	44,167.6	284	186+00 (B-47)	2,312,361.3	36,892.6	204
044+25 (I-12)	2,300,754.6	44,090.2	276	190+00 (B-48)	2,312,725.8	36,727.8	204
045+07 (B-13)	2,300,751.4	44,007.0	268	194+00 (B-49)	2,313,090.2	36,562.9	204
046+00 (I-13)	2,300,784.9	43,920.7	260	198+00 (B-50)	2,313,454.7	36,398.1	204
046+89 (B-14)	2,300,813.7	43,836.0	251	202+00 (B-51)	2,313,819.2	36,233.2	204
049+00 (H-13)	2,300,881.5	43,636.5	247	206+00 (B-52)	2,314,183.6	36,068.4	204
050+50 (I-14)	2,300,913.5	43,541.9	247	210+00 (B-53)	2,314,548.1	35,903.5	204
051+00 (J-14)	2,300,945.8	43,447.1	247	214+00 (B-54)	2,314,912.5	35,738.7	204
052+64 (B-15)	2,300,998.3	43,292.1	243	218+00 (B-55)	2,315,277.0	35,573.8	204
054+00 (I-15)	2,301,042.2	43,163.0	243	· · · · · ·	East Beac		
	South Beac			224+80 (EB-01)	2,315,748.8	36,063.3	90
056+56 (B-16)	2,301,148.7	42,933.8	233	234+80 (EB-02)	2,315,748.8	37,063.3	90
060+51 (B-17)	2,301,399.6	42,628.3	230	244+80 (EB-03)	2,315,748.8	38,063.3	90
065+50 (B-18)	2,301,716.0	42,243.2	229	254+80 (EB-04)	2,315,748.8	39,063.3	90
069+46 (B-19)	2,301,967.6	41,937.0	227	264+80 (EB-05)	2,315,748.8	40,063.3	90
073+39 (B-20)	2,302,246.1	41,660.5	223	274+80 (EB-06)	2,315,748.8	41,063.3	90
076+37 (B-21)	2,302,609.2	41,320.5	222	284+80 (EB-07)	2,315,748.8	42,063.3	90

 Table 2.1: Bald Head Island baseline stationing and beach monitoring profile locations.

Survey Date	Surveyor	Comments
1999 Nov.	Brunswick Surveying., Inc.	16 months pre-construction (2001 disposal)
2000 Nov.	Brunswick Surveying., Inc.	4 months pre-construction (2001 disposal)
2001 Aug.	Brunswick Surveying., Inc.	1 month post-construction (2001 disposal)
2002 July	Brunswick Surveying., Inc.	12 months post-construction (2001 disposal)
2002 Dec.	Brunswick Surveying., Inc.	17 months post-construction (2001 disposal)
2003 May	Brunswick Surveying., Inc.	22 months post-construction (2001 disposal)
2003 Oct.	McKim & Creed	27 months post-construction (2001 disposal)
2004 Apr.	McKim & Creed	33 months post-construction (2001 disposal)
2004 Oct.	McKim & Creed	39 months post-construction (2001 disposal)
2005 Apr.	McKim & Creed	3 months post-construction (2004/05 disposal)
2005 Nov.	McKim & Creed	10 months post-construction (2004/05 disposal)
2006 Apr	McKim & Creed	15 months post-construction (2004/05 disposal)
2006 Nov.	McKim & Creed	22 months post-construction (2004/05 disposal)
2007 June	McKim & Creed	2 months post-construction (2007 disposal)
2007 Nov.	McKim & Creed	7 months post-construction (2007 disposal)
2008 May	McKim & Creed	13 months post-construction (2007 disposal)
2008 Nov.	McKim & Creed	19 months post-construction (2007 disposal)
2009 May	McKim & Creed	25 months post-construction (2007 disposal)
2009 Sept.	Gahagan & Bryant	Survey required by dredge Contractor.
2010 May	McKim & Creed	2 months post-renourishment (09/10)
2010 Sept.	McKim & Creed	6 months post-renourishment (09/10)
2011 May	McKim & Creed	14 months post-renourishment (09/10)
2011 Sept.	McKim & Creed	18 months post-renourishment (09/10)
2012 May	McKim & Creed	26 months post-renourishment (09/10)
2012 Nov.	McKim & Creed	32 months post-renourishment (09/10)
2013 May	McKim & Creed	38 months post-renourishment (09/10)
2013 Nov.	McKim & Creed	44 months post-renourishment (09/10)
2014 May	McKim & Creed	50 months post-renourishment (09/10)
2014 Nov.	McKim & Creed	56 months post-renourishment (09/10)
2015 April	McKim & Creed	1 month post-construction (2015 Disposal)
2015 Nov.	McKim & Creed	8 months post-construction (2015 Disposal)
2016 April	McKim & Creed	13 months post-construction (2015 Disposal)
2016 Oct.	McKim & Creed	19 months post-construction (2015 Disposal)
2017 May	McKim & Creed	26 months post-construction (2015 Disposal)
2018 Nov.	McKim & Creed	32 months post-construction (2015 Disposal)
2018 May	McKim & Creed	38 months post-construction (2015 Disposal)
2019 Sept.	McKim & Creed	Post-Hurricane Florence
2019 May	McKim & Creed	2 months post-renourishment (18/19)
2019 Nov	McKim & Creed	8 months post-renourishment (18/19)
2020 May	McKim & Creed	14 months post-renourishment (18/19)
2021 May	McKim & Creed	26 months post-renourishment (18/19)

 Table 2.2: Bald Head Island monitoring surveys collected as of May 2021.

<u>MHWL SURVEYS</u> As part of the permit required monitoring for the terminal groin project completed in late 2015, post-construction MHWL surveys were initiated in November 2015. Each survey was specified to begin at the Marina entrance (Sta. 0+00) and extend to St. 75+00, about 3,000 ft eastward of the terminal groin head. On an annual basis, surveys are to be intercompared to assess both updrift fillet conditions and the location of the downdrift shoreline fronting the Cape Fear River.

2.2 Bald Head Creek Borrow Site Surveys

The Bald Head Creek borrow site and adjacent areas utilized for the 2017 Shore Stabilization Project constructed by Marcol Dredging were monitored at 6 months and thereafter annually for the following 3 years. **Table 2.3** summarizes selected surveys collected to date. *No survey was required or conducted during the current monitoring year*. The purpose of the monitoring is to document hydrographic changes throughout the borrow site and in particular areas which were "over-dredged" by the Contractor. Of specific interest to State and Federal regulatory agencies has been the rate of recovery and the composition of the material that infills the area(s) excavated by hydraulic dredge below that addressed by permit. In addition to annual surveys, limited grab samples and sediment analysis are performed by the firm LMG. The Marcol March 2017 AD Survey is considered as the "baseline condition". As of May 2019, physical monitoring was deemed complete.

Borrow Site Survey Date	Comment
March 2012	After Dredge (AD) Survey (11/12 Project)
January 2013	10 Months Post-Dredge (11/12 Project)
December 2013	21 Months Post-Dredge (11/12 Project)
April 2015	37 Months Post-Dredge (11/12 Project)
April 2016	49 Months Post-Dredge (11/12 Project)
November 2016	Before Dredge (BD) Survey (16/17 Project)
March 2017	After Dredge (AD) Survey (16/17 Project)
November 2017	7 Months Post-Dredge (11/12 Project)
May 2018	14 Months Post-Dredge (11/12 Project)
May 2019 ⁽¹⁾	26 Months Post-Dredge (11/12 Project)

Table 2.3: Selected Bald Head Creek borrow site surveys collected as of May 2021.

(1) Last monitoring survey required.

2.3 Jay Bird Shoal Borrow Site Surveys

Permits for the most recent beach renourishment project constructed by the Village in 2019 necessitate the resurveying of the Jay Bird Shoal borrow site as part of the annual island-wide monitoring program. **Table 2.4** summarizes the borrow site surveys conducted to date. Specifically, borrow site surveys are required both pre- and post-excavation, as well as at 12-, 24- and 36-months and biennially thereafter. The area typically surveyed is 400-acres \pm which includes a buffer area outside the "permitted" limits of work.

Borrow Site Survey Date	Comment
October 2009	Before Dredge (BD) Survey (09/10 Project)
March 2010	After Dredge (AD) Survey (09/10 Project)
May 2011	14 Months Post-Dredge (09/10 Project)
May 2012	26 Months Post-Dredge (09/10 Project)
May 2013	38 Months Post-Dredge (09/10 Project)
April 2015	61 Months Post-Dredge (09/10 Project)
May 2017	86 Months Post-Dredge (09/10 Project)
November 2017	92 Months Post-Dredge (09/10 Project)
May 2018	98 Months Post-Dredge (09/10 Project)
September 2018	98 Months Post-Dredge (09/10 Project)
December 2018	Before Dredge (BD) Survey (19 Project)
March 2019	After Dredge (AD) Survey (19 Project)
May 2020	14 Months Post Project (19 Project)
May 2021	26 Months Post Project (19 Project)

Table 2.4: Jay Bird Shoal borrow site surveys collected as of May 2021.

2.4 Orthorectified Aerial Photography

In addition to the beach profile surveys, digital color aerial photography of the island's shoreline has been acquired at a minimum, annually by Greenman-Pedersen, Inc.⁶ **Table 2.5** summarizes the aerial photography collected to date as part of the monitoring program. Reproductions of the three most recent aerial photography sets (May 2021, November 2020 and May 2020) are presented in **Appendices B**, **C** and **D**, respectively.

⁶ Greenman-Pederson, Inc.; 3909 Wrightsville Ave. Suite 200; Wilmington, NC 28403.

Photo Date			
Year	Month	Day	Comment
2001	September	NA	2 months post-construction (2001 disposal)
2002	November	14	16 months post-construction (2001 disposal)
2003	April	NA	21 months post-construction (2001 disposal)
2004	January	NA	30 months post-construction (2001 disposal)
2004	May	NA	34 months post-construction (2001 disposal)
2004	October	NA	39 months post-construction (2001 disposal)
2005	May	NA	4 months post-construction (2004/05 disposal)
2005	November	NA	10 months post-construction (2004/05 disposal)
2006	April	NA	15 months post-construction (2004/05 disposal)
2006	October	NA	21 months post-construction (2004/05 disposal)
2007	May	20	1month post-construction (2007 disposal)
2008	May	13	13 months post-construction (2007 disposal)
2009	January	14	21 months post-construction (2007 disposal)
2009	May	31	25 months post-construction (2007 disposal)
2009	August	26	3 months pre-renourishment (09/10)
2010	April	NA	1 month post-renourishment (09/10)
2011	April	NA	13 months post-nourishment (09/10)
2012	May	NA	26 months post-nourishment (09/10)
2012	December	14	33 months post-nourishment (09/10)
2013	May	14	38 months post-nourishment (09/10)
2013	November	14	44 months post-nourishment (09/10)
2014	May	23	50 months post-nourishment (09/10)
2014	November	03	56 months post-nourishment (09/10)
2015	March	29	Post-construction (2015 Disposal)
2015	August	9	5 months post-construction (2015 Disposal)
2015	November	29	Post-terminal groin construction
2016	April	3	4 months post-construction (T.G.)
2016	October	13	Post-Hurricane Matthew
2017	April	14	5 months Post-Hurricane Matthew
2017	November	27	24 months post-construction (T.G.)
2018	April	19	29 months post-construction (T.G.)
2018	October	14	Post-Hurricane Florence
2019	April	10	1 month post-renourishment (18/19)
2019	Nov	13	8 months post-renourishment (18/19)
2020	May	15	13 months post-renourishment (18/19)
2020	Nov	19	20 months post-renourishment (18/19)
2021	May	7	25 months post-renourishment (18/19)

Table 2.5: Bald Head Island monitoring aerial photography collected as of May 2021.

3.0 MONITORING (SURVEY) RESULTS

3.1 Methodology

For purposes of analysis and discussion, the Bald Head Island monitoring baseline is qualitatively broken into seven (7) shoreline segments, or zones of interest, with significantly varying physiographic characteristics as follows:

- Station -018+72 to -003+00 "Row Boat Row"
- Station -001+60 to 028+00 "West Beach"
- Station 028+00 to 046+00 "The Point" North of Terminal Groin
- Station 046+00 to 056+56 "The Point" South of Terminal Groin
- Station 056+56 to 214+00 "South Beach"⁷
- Station 214+00 to 224+80 "Cape Fear Point"⁸
- Station 224+80 to 284+80 "East Beach"

These zones differ slightly from the shoreline segments used monitoring reports prior to 2015 (OAI 2015). More specifically, "the Point" is now divided into two areas rather than one, based upon the location of the recently completed terminal groin. The update is intended to more accurately capture the influence of that structure on the physical processes along the Bald Head Island shoreline. Additionally, the "Row Boat Row" reach was added to the monitoring analysis with the initial monitoring surveys along this reach completed in November 2015.

Alongshore volume changes were calculated using an average end-area method, where the cross-sectional areas are determined by comparing beach profiles at each beach monitoring station above several different vertical datums. This approach allows evaluation of beach changes at different elevations along the project in addition to the total profile.

Average shoreline position changes were spatially weighted based upon the distance between stations due to the non-uniform alongshore spacing of survey monuments.

⁷ East of Sta. 214+00, the remaining 400 ft of surveyed Atlantic oceanfront shoreline becomes part of Cape Fear Point and is not included in the South Beach analysis due to its highly dynamic nature.

⁸ The general condition of the Cape Fear spit is qualitatively monitored primarily through controlled aerial photography. This depositional feature is routinely subject to episodic periods of accretion and erosion resulting from eventual detachment via tidal channel breakthrough during storms. It is likewise influenced by beach fill activities and sediment added to the littoral system of South Beach as well as storm waves originating from the east or southeast.

3.2 Year 20: Monitoring Program (May 2020 – November 2020 – May 2021)

The May 2020 to May 2021 monitoring period represents the twentieth year of measured shoreline change following completion of the initial 2001 Federal +1.849 Mcy beach disposal at Bald Head Island. For compliance purposes, the May 2021 survey represents the 2nd year following completion of the +1.1 Mcy beach fill constructed by the Village of Bald Head Island in the winter of 2018/2019 at South Beach. This survey period also serves to document the April 2021 completion of a 1.61 Mcy Federal beach disposal project at S. Beach.

Volume changes between condition surveys were computed using the average end-area method above the mean high water line (MHWL; +2.51 ft-NGVD) and the assumed typical depth of closure (-16.0 ft-NGVD). **Tables 3.1** through **3.3** list the computed changes along the Bald Head Island shoreline for the May 2020 – November 2020 – May 2021 survey intervals. **Figures 3.1**, **3.2** and **3.3** depict the cumulative and local volume changes for the same intervals. Changes in shoreline position at each station were computed at the MHWL and the seaward edge of berm (+6 ft-NGVD contour). The results are summarized in **Tables 3.4** and **3.5** and graphically depicted in **Figures 3.4** and **3.5** (relative to their *November 2000* pre-disposal locations).

3.3 West Beach, "The Point" and South Beach: Discussion

3.3.1 Survey Period: May 2020 to November 2020 (Hurricane Dorian)

This survey period represents the second monitoring period following completion of the +1.1 Mcy 2018/19 beach fill. The May 2020 and November 2020 surveys were performed approximately 14 and 20 months, respectively, following project completion in March 2019. This six (6) month survey period also reflects the prior impact of Hurricane Isaias (early October 2020).

As depicted in **Figure 3.1** and **Table 3.1**, the island-wide *net* shoreline volume change trend for this period was erosional with -192,400 cy (-15.9 cy/ft) of loss, mol. over the 6 month span above -16 ft-NGVD. Similarly, above the MHWL, the shoreline lost -31,000 cy. Consistent with the volume losses above the MHWL, the berm receded by a spatially weighted average⁹ of roughly -26 feet while the MHWL retreated by -24 feet. However, subreaches of the monitored shoreline experienced varied changes over this monitoring period.

⁹ Due to the irregular spacing of the profile stations, the average shoreline changes are weighted based upon the distance between stations and calculated using an average end method.

	Volume Change						Volume	Change				
				Above	Above					Above	Above	,
	Start Station	End Station	Reach (FT)	+2.51 (FT)	-16 (FT)		Start Station	End Station	Reach (FT)	+2.51 (FT)	-16 (FT)	
	Jetty	000+00	160	-400	-1,000		056+56	060+51	423	-300	-4,200	
	000+00	004+00	400	-600	-2,100		060+51	065+50	510	-2,400	-9,500	
	004+00	008+00	400	-300	-1,100		065+50	069+46	423	-2,100	-7,000	
	008+00	010+00	200	-200	-300		069+46	073+39	442	-1,400	-4,700	
ch	010+00	012+00	200	-200	-500		073+39	076+37	516	-100	-2,300	
Bea	012+00	014+00	200	-100	-400		076+37	084+16	611	-100	-300	
West Beach	014+00	016+00	200	-100	-400		084+16	088+23	471	-1,700	-3,800	
W.	016+00	018+00	200	-200	-300		088+23	092+15	455	-2,400	-5,100	
	018+00	020+00	200	0	+400		092+15	097+10	536	-1,200	-5,800	
	020+00	024+00	400	-600	-3,000		097+10	102+08	525	0	-1,900	
	024+00	028+00	400	-2,900	-9,600		102+08	106+00	436	-600	+400	
		Subtotal	2,960	-5,600	-18,300		106+00	110+00	400	-1,400	-3,700	
	028+00	032+00	395	-4,300	-12,000		110+00	114+00	388	-900	-2,000	
	032+00	034+00	200	-2,100	-5,900		114+00	118+00	407	-900	-2,100	
in)	034+00	036+00	210	-700	+4,900		118+00	122+00	413	-2,000	-3,400	
Gro	036+00	038+00	230	+2,100	+13,600		122+00	126+00	405	-2,500	-5,500	
of (038+00	039+60	230	+2,000	+3,100		126+00	130+00	405	-2,100	-6,500	
Point (North of Groin)	039+60	041+50	220	0	-6,300		130+00	134+00	398	-1,900	-4,500	\sim
(N0	041+50	043+47	220	-1,600	-11,900		134+00	138+00	401	-1,800	-3,500	out
int (043+47	044+25	190	-3,100	-15,700		138+00	142+00	400	-700	-2,600	h B
Poj	044+25	045+07	190	-3,500	-15,000		142+00	146+00	400	-900	-4,800	South Beach
	045+07	046+00	200	-2,500	-7,000		146+00	150+00	399	-500	-2,400	h
		Subtotal	2,285	-13,700	-52,200		150+00	154+00	385	+600	+2,700	
	046+00	046+89	200	-600	-3,700		154+00	158+00	383	+300	+2,000	
uth of Groin)	046+89	049+00	250	-100	-8,200		158+00	162+00	386	+300	+700	
f Gr	049+00	050+50	100	-100	-3,600		162+00	166+00	393	+700	-6,500	
th of	050+50	051+00	100	-200	-3,100		166+00	170+00	394	+700	-5,400	
	051+00	052+64	240	-200	-4,800		170+00	174+00	400	+2,400	+3,800	
Point (So	052+64	054+00	135	+300	-900		174+00	178+00	400	+2,700	+1,200	
Poi	054+00	056+56	380	+900	-700		178+00	182+00	400	+600	-4,600	
		Subtotal	1,405	0	-25,000		182+00	186+00	400	+1,300	+400	
	Note: Elev	ations are re	eferenced	to NGVD	1929.		186+00	190+00	400	+1,000	+5,200	
							190+00	194+00	400	-900	-9,100	
								198+00	400	-300	-4,800	
1							198+00	202+00	400	0	+3,400	
							202+00 206+00	206+00	400	+2,400	+4,300	
1								210+00	400	+4,000	+4,900	
							210+00	214+00	400	+400	-9,900	
1								Subtotal	16,105	-11,700	-96,900	
							Bald He	ead Total	22,755	-31,000	-192,400	

 Table 3.1: Bald Head Island shoreline volume change (May 2020 to November 2020).

Siart End Rec. Above (FT) Above (FT) Siart End Res. Above (FT) Above (FT) Above (FT) 1ety 000+00 160 -100 -2,600 056+56 060+51 423 +15,800 +22,400 000+00 010+00 200 +200 -3,600 066+51 065+50 510 +11,500 +33,600 000+00 012+00 200 +200 -300 069+46 (73+39 +42 +14,500 +33,000 012+00 012+00 200 +200 00 77+37 516 +16,500 +44,45,00 012+00 014+00 200 +100 +100 07+37 084+16 611 +18,000 +38,400 018+00 020+0 440 +500 088+23 092+15 455 +16,600 +38,100 020+00 020+0 +200 200 102+08 102+08 525 +14,700 +33,100 022+00 032+00 <	Table 5.2: Dalu Heau			Volume Change					Volume Change				
Start Sution End Full Reach Station +2.51 (FT) -1.6 (FT) Station (FT) Fnd Station Reach Full +2.30 (FT) -1.6 (FT) Station (FT) -1.6 (FT) -1.6 (FT													
Jetty 000+00 160 -100 -2,600 056+56 060+51 423 +5,800 +22,400 000+00 000+00 0400 +500 -700 065+50 510 +11,500 +37,600 000+00 008+00 010+00 200 +200 -300 069+46 073+39 442 +14,500 +38,600 010+00 012+00 200 +200 0 075+37 516 +16,500 +444,500 014+00 116+00 200 +400 +500 084+16 611 +15,000 +38,100 016+00 118+00 200 +400 +200 092+15 97+10 53 +16,300 +38,100 028+00 028+00 400 -2,500 -13,700 102+08 106+00 436 +13,800 +22,500 028+00 032+00 230 -2,300 -5,400 110+00 144+00 388 +13,600 +25,500 028+00 036+00 230					+2.51	-16					+2.51	-16	
000-00 004+00 400 +200 -3.600 060+51 065+50 510 +11,500 +37,600 008+00 010+00 200 +200 -300 069+46 073+39 442 +14,500 +33,600 010+00 012+00 200 +200 0 073+39 076+37 516 +16,500 +44,500 012+00 014+00 200 +100 +100 076+37 084+16 611 +18,000 +38,400 016+00 018+00 200 +100 +500 088+23 92+15 455 +16,500 +38,100 016+00 020+00 200 +600 +200 092+16 102+08 525 +14,700 +33,100 024+00 220+0 400 -13,700 102+08 106+00 436 +13,800 +25,500 032+00 334+00 220 +1,400 +9,300 110+00 114+00 188 +13,600 +25,500 038+00 230				<i>(</i>									
004+00 008+00 400 +500 -700 008+00 010+00 200 +200 -300 065+46 073+39 442 +13,100 +38,600 012+00 012+00 200 +200 0 073+39 076+37 516 +16,500 +44,500 012+00 014+00 016+00 200 +400 +500 088+16 088+23 471 +15,900 +38,400 016+00 018+00 200 +400 +200 097+10 536 +16,300 +38,400 028+00 022+00 200 +100 -8,100 097+10 102+08 106+00 113,700 +24,800 028+00 022+00 200 +1,40 9,300 102+08 106+00 413 +13,200 +23,800 028+00 032+00 230 -100 -2,400 110+00 140+00 181,10 +13,700 +24,800 038+00 230 -2,800 -3,400 138+00 144,700<											-	-	
008+00 010+00 200 +200 -300 069+46 073+39 442 +14,500 +43,000 010+00 012+00 200 +200 0 073+39 076+37 516 +16,500 +44,500 012+00 014+00 200 +100 +100 073+39 076+37 516 +16,500 +44,500 016+00 018+00 200 +400 +200 084+16 611 +18,000 +38,100 016+00 018+00 200 +600 +200 082+15 097+10 536 +16,300 +38,100 020+00 024+00 400 -2,500 +13,700 102+08 106+00 436 +13,800 +25,100 028+00 032+00 395 -2,300 5,800 110+00 114+00 188,80 +25,200 +38,800 034+00 230 -100 -2,400 122+00 126+00 405 +15,600 +32,600 034+00 038+00 230 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td></td> <td></td>						-					-		
010+00 012+00 200 +200 0 012+00 014+00 200 +100 +100 +100 +100 +100 +100 +100 +137 084+16 611 +18,000 +38,400 014+00 016+00 200 +400 +500 084+16 684+23 471 +15,900 +38,100 018+00 020+00 200 +600 +200 092+15 97+10 536 +16,300 +38,100 020+00 024+00 400 -100 -8,100 097+10 102+08 525 +14,700 +33,100 024+00 032+00 395 -2,300 -5,800 110+00 114+00 406 +13,800 +25,100 032+00 034+00 200 +1,400 +9,300 114+00 118+00 407 +15,100 +28,800 034+00 038+00 230 -2,600 -6,600 134+00 398 +16,700 +39,400 04+150 040											-	-	
OI2+00 OI4+00 200 +100 +100 +100 OI4+00 OI6+00 200 +400 +500 084+16 611 +18,000 +38,400 OI6+00 018+00 200 +400 +500 088+23 92+15 455 +16,600 +38,100 OI6+00 020+00 224+00 400 -100 -8,100 092+15 957+10 536 +16,300 +38,100 O24+00 028+00 400 -2,500 -13,700 106+00 104+00 436 +13,800 +25,100 OX4+00 036+00 210 +2,500 +11,10 106+00 104+00 400 +3,600 +24,800 O34+00 036+00 210 +2,500 +11,10 118+00 122+00 413 +15,200 +32,600 O34+00 036+00 230 -100 -2,400 122+00 126+00 400 +16,00 +32,600 O34+00 039+60 41+50 220 -4,000													
018+00 020+00 200 +600 +200 020+00 024+00 028+00 400 -100 -8,100 024+00 028+00 400 -2,500 -13,700 102+08 106+00 436 +13,800 +25,100 028+00 032+00 034+00 200 +2,000 -27,000 110+00 110+00 400 +13,700 +22,800 032+00 034+00 200 +1,400 +9,300 110+00 110+00 400 +13,600 +25,500 032+00 034+00 230 -2,000 +100 118+00 122+00 413 +15,000 +32,600 038+00 039+60 230 -2,800 -9,400 136+00 138+00 400 +16,700 +33,400 041+50 023+47 220 -2,600 -6,600 134+00 142+00 440 +16,700 +33,400 044+25 045+07 190 -700 +3,600 142+00 146+00 400	ach											-	
018+00 020+00 200 +600 +200 020+00 024+00 028+00 400 -100 -8,100 024+00 028+00 400 -2,500 -13,700 102+08 106+00 436 +13,800 +25,100 028+00 032+00 034+00 200 +2,000 -27,000 110+00 110+00 400 +13,700 +22,800 032+00 034+00 200 +1,400 +9,300 110+00 110+00 400 +13,600 +25,500 032+00 034+00 230 -2,000 +100 118+00 122+00 413 +15,000 +32,600 038+00 039+60 230 -2,800 -9,400 136+00 138+00 400 +16,700 +33,400 041+50 023+47 220 -2,600 -6,600 134+00 142+00 440 +16,700 +33,400 044+25 045+07 190 -700 +3,600 142+00 146+00 400	Be												
018+00 020+00 200 +600 +200 020+00 024+00 028+00 400 -100 -8,100 024+00 028+00 400 -2,500 -13,700 102+08 106+00 436 +13,800 +25,100 028+00 032+00 034+00 200 +2,000 -27,000 110+00 110+00 400 +13,700 +22,800 032+00 034+00 200 +1,400 +9,300 110+00 110+00 400 +13,600 +25,500 032+00 034+00 230 -2,000 +100 118+00 122+00 413 +15,000 +32,600 038+00 039+60 230 -2,800 -9,400 136+00 138+00 400 +16,700 +33,400 041+50 023+47 220 -2,600 -6,600 134+00 142+00 440 +16,700 +33,400 044+25 045+07 190 -700 +3,600 142+00 146+00 400	'est										-	,	
020+00 024+00 400 -100 -8,100 024+00 028+00 400 -2,500 -13,700 Subtotal 2,960 +200 -27,000 102+08 106+00 436 +13,800 +25,100 028+00 032+00 3295 -2,300 -5,800 110+00 110+00 400 +13,700 +24,800 032+00 034+00 200 +1,400 +9,300 110+00 118+00 122+00 413 +15,200 +30,800 032+00 038+00 230 -100 -2,400 118+00 122+00 400 +15,100 +32,600 038+00 039+60 230 -2,600 +10,00 134+00 134+00 398 +16,700 +33,600 041+50 043+47 244+25 190 -500 +1,200 +40,800 134+00 144+00 401 +16,500 +43,600 044+25 044+25 190 -500 +3,700 138+00 142+00 400	M										-		
024+00 028+00 400 -2,500 -13,700 102+08 106+00 436 +13,800 +25,100 Subtotal 2,960 +200 -27,000 106+00 110+00 436 +13,800 +25,100 028+00 032+00 395 -2,300 -5,800 110+00 114+00 388 +13,600 +25,500 034+00 036+00 210 +2,500 +1,110 118+00 122+00 413 +15,200 +30,800 038+00 039+60 230 -2,800 -9,400 122+00 126+00 405 +16,800 +37,200 039+60 041+50 220 -4,000 -10,700 130+00 134+00 384 14,700 +44,800 041+50 043+47 244 -500 +1,300 +12,200 124+00 142+00 400 +16,100 +41,200 043+47 044+25 190 -500 +1,300 +12,200 142+00 140+00 400 +15,700 +			020+00								+16,300	+38,100	
Subtocal 2,960 +200 -27,000 106+00 110+00 400 +13,700 +24,800 028+00 032+00 395 -2,300 -5,800 110+00 110+00 400 +13,700 +24,800 032+00 034+00 200 +1,400 +9,300 1114+00 184:00 407 +15,100 +28,800 034+00 036+00 230 -2,800 +9,400 118+00 407 +15,000 +32,600 038+00 039+60 230 -2,800 -9,400 130+00 30400 398 +16,700 +39,400 041+50 043+47 220 -2,600 -6,600 134+00 138+00 401 +16,500 +40,800 044+25 045+07 190 -700 +3,600 142+00 146+00 400 +15,700 +43,600 045+07 046+89 200 -400 +5,900 158+00 383 +14,300 +43,500 046+89 040 +1,200			024+00	400		-		097+10	102+08	525	+14,700	+33,100	
O28+00 032+00 395 -2,300 -5,800 032+00 034+00 200 +1,400 +9,300 034+00 036+00 210 +2,500 +11,10 036+00 038+00 230 -100 -2,400 114+00 118+00 407 +15,100 +28,800 036+00 038+00 230 -100 -2,400 1122+00 413 +15,200 +30,800 038+00 039+60 230 -2,800 -9,400 126+00 130+00 405 +16,800 +32,600 039+60 041+50 220 -4,000 -10,700 130+00 134+00 398 +16,700 +39,400 041+50 043+47 244-25 190 -500 +1,200 142+00 400 +16,100 +41,200 045+07 046+00 200 -900 +3,700 122+00 142+00 400 +15,700 +43,600 046+00 050+50 100 +900 +5,500		024+00	028+00	400	-2,500	-13,700		102+08	106+00	436		+25,100	
O32+00 O34+00 200 +1,400 +9,300 114+00 118+00 407 +15,100 +28,800 034+00 036+00 210 +2,500 +11,10 118+00 122+00 413 +15,200 +30,800 036+00 038+00 230 -100 -2,400 122+00 126+00 405 +15,600 +32,600 039+60 041+50 220 -4,000 -10,700 134+00 134+00 398 +16,700 +39,400 041+50 043+47 220 -2,600 -6,600 134+00 138+00 401 +16,500 +40,800 044+25 045+07 190 -700 +3,600 142+00 146+00 400 +16,100 +41,200 045+07 046+00 200 -900 +3,700 138+00 142+00 146+00 146+00 383 +14,800 +44,700 046+89 049+00 250 +1,300 +12,200 158+00 158+00 383 +14,300<			Subtotal	2,960	+200	-27,000		106+00	110+00	400	+13,700	+24,800	
Understand 036+00 210 +2,500 +11,10 036+00 038+00 230 -100 -2,400 118+00 122+00 413 +15,200 +30,800 038+00 039+60 230 -2,800 -9,400 122+00 126+00 405 +15,600 +32,600 039+60 041+50 220 -4,000 -10,700 130+00 134+00 398 +16,700 +39,400 041+50 043+47 220 -2,600 -6,600 134+00 138+00 401 +16,500 +40,800 044+25 045+07 190 -700 +3,600 142+00 400 +15,700 +43,600 045+07 046+00 200 -900 +3,700 154+00 385 +14,200 +42,400 046+89 049+00 250 +13,300 +12,200 158+00 158+00 385 +14,200 +44,500 050+50 051+00 100 +1,200 +5,500 158+00 162+00 </td <td></td> <td>028+00</td> <td>032+00</td> <td>395</td> <td>-2,300</td> <td>-5,800</td> <td></td> <td>110+00</td> <td>114+00</td> <td>388</td> <td>+13,600</td> <td>+25,500</td> <td></td>		028+00	032+00	395	-2,300	-5,800		110+00	114+00	388	+13,600	+25,500	
036+00 038+00 230 -100 -2,400 038+00 039+60 230 -2,800 -9,400 039+60 041+50 220 -4,000 -10,700 041+50 043+47 220 -2,600 -6,600 043+47 044+25 190 -500 +1,200 043+47 044+25 190 -500 +1,200 044+25 045+07 190 -700 +3,600 045+07 046+00 200 -900 +3,700 046+00 046+89 200 -400 +5,900 046+89 049+00 250 +1,300 +12,200 050+50 051+00 100 +12,000 +55,000 051+00 052+64 240 +3,000 +12,600 158+00 182+00 400 +15,000 +46,900 158+00 055+56 380 +2,800 +14,800 158+00 182+00 400 +15,000 +46,900 <t< td=""><td></td><td>032+00</td><td>034+00</td><td>200</td><td>+1,400</td><td>-</td><td></td><td>114+00</td><td>118+00</td><td>407</td><td>+15,100</td><td>+28,800</td><td></td></t<>		032+00	034+00	200	+1,400	-		114+00	118+00	407	+15,100	+28,800	
1000 033+00 039+60 230 -2,800 -9,400 126+00 130+00 405 +16,800 +37,200 039+60 041+50 220 -4,000 -10,700 130+00 134+00 398 +16,700 +39,400 041+50 043+47 220 -2,600 -6,600 134+00 138+00 401 +16,500 +40,800 043+47 044+25 190 -500 +1,200 138+00 401 +16,700 +39,400 044+25 045+07 190 -700 +3,600 138+00 142+00 400 +15,700 +43,600 046+00 200 -900 +3,700 150+00 154+00 385 +14,200 +44,200 046+00 046+89 200 -400 +5,900 154+00 158+00 386 +14,700 +43,500 050+50 051+00 100 +1,200 +5,500 162+00 386 +15,500 +52,200 054+00 056+56	in)	034+00	036+00	210	+2,500	+11,10		118+00	122+00	413	+15,200	+30,800	
1000 033+00 039+60 230 -2,800 -9,400 126+00 130+00 405 +16,800 +37,200 039+60 041+50 220 -4,000 -10,700 130+00 134+00 398 +16,700 +39,400 041+50 043+47 220 -2,600 -6,600 134+00 138+00 401 +16,500 +40,800 043+47 044+25 190 -500 +1,200 138+00 401 +16,700 +39,400 044+25 045+07 190 -700 +3,600 138+00 142+00 400 +15,700 +43,600 046+00 200 -900 +3,700 150+00 154+00 385 +14,200 +44,200 046+00 046+89 200 -400 +5,900 154+00 158+00 386 +14,700 +43,500 050+50 051+00 100 +1,200 +5,500 162+00 386 +15,500 +52,200 054+00 056+56	Gro	036+00	038+00	230	-100	-2,400		122+00	126+00	405	+15,600	+32,600	
045+07 046+00 200 -900 +3,700 146+00 150+00 399 +14,800 +44,700 200 046+00 046+89 200 -400 +5,900 150+00 154+00 385 +14,200 +42,400 046+89 049+00 250 +1,300 +12,200 158+00 383 +14,300 +43,500 049+00 050+50 051+00 100 +1,200 +5,600 162+00 386 +14,700 +44,500 051+00 052+64 240 +3,000 +12,600 166+00 393 +15,500 +52,200 052+64 054+00 135 +800 +5,000 174+00 178+00 400 +14,900 +46,300 054+00 056+56 380 +2,800 +14,800 170+00 174+00 178+00 400 +13,400 +40,600 182+00 186+00 400 +11,700 +36,500 190+00 400 +11,700 +36,500 190+00	of	038+00	039+60	230	-2,800	-9,400		126+00	130+00	405	+16,800	+37,200	
045+07 046+00 200 -900 +3,700 146+00 150+00 399 +14,800 +44,700 200 046+00 046+89 200 -400 +5,900 150+00 154+00 385 +14,200 +42,400 046+89 049+00 250 +1,300 +12,200 158+00 383 +14,300 +43,500 049+00 050+50 051+00 100 +1,200 +5,600 162+00 386 +14,700 +44,500 051+00 052+64 240 +3,000 +12,600 166+00 393 +15,500 +52,200 052+64 054+00 135 +800 +5,000 174+00 178+00 400 +14,900 +46,300 054+00 056+56 380 +2,800 +14,800 170+00 174+00 178+00 400 +13,400 +40,600 182+00 186+00 400 +11,700 +36,500 190+00 400 +11,700 +36,500 190+00	rth	039+60	041+50	220	-4,000	-10,700		130+00	134+00	398	+16,700	+39,400	
045+07 046+00 200 -900 +3,700 146+00 150+00 399 +14,800 +44,700 200 046+00 046+89 200 -400 +5,900 150+00 154+00 385 +14,200 +42,400 046+89 049+00 250 +1,300 +12,200 158+00 383 +14,300 +43,500 049+00 050+50 051+00 100 +1,200 +5,600 162+00 386 +14,700 +44,500 051+00 052+64 240 +3,000 +12,600 166+00 393 +15,500 +52,200 052+64 054+00 135 +800 +5,000 174+00 178+00 400 +14,900 +46,300 054+00 056+56 380 +2,800 +14,800 170+00 174+00 178+00 400 +13,400 +40,600 182+00 186+00 400 +11,700 +36,500 190+00 400 +11,700 +36,500 190+00	No	041+50	043+47	220	-2,600	-6,600		134+00	138+00	401	+16,500	+40,800	out
045+07 046+00 200 -900 +3,700 146+00 150+00 399 +14,800 +44,700 200 046+00 046+89 200 -400 +5,900 150+00 154+00 385 +14,200 +42,400 046+89 049+00 250 +1,300 +12,200 158+00 383 +14,300 +43,500 049+00 050+50 051+00 100 +1,200 +5,600 162+00 386 +14,700 +44,500 051+00 052+64 240 +3,000 +12,600 166+00 393 +15,500 +52,200 052+64 054+00 135 +800 +5,000 174+00 178+00 400 +14,900 +46,300 054+00 056+56 380 +2,800 +14,800 170+00 174+00 178+00 400 +13,400 +40,600 182+00 186+00 400 +11,700 +36,500 190+00 400 +11,700 +36,500 190+00	nt (043+47	044+25	190	-500	+1,200		138+00	142+00	400	+16,100	+41,200	ΉB
045+07 046+00 200 -900 +3,700 146+00 150+00 399 +14,800 +44,700 Subtotal 2,285 -10,000 -6,000 150+00 385 +14,200 +42,400 046+00 046+89 200 -400 +5,900 154+00 385 +14,200 +42,400 046+89 049+00 250 +1,300 +12,200 158+00 383 +14,300 +43,500 046+89 049+00 250 +1,300 +12,200 158+00 386 +14,700 +44,500 050+50 051+00 100 +12,200 +5,600 166+00 393 +15,700 +50,700 051+00 052+64 240 +3,000 +12,600 166+00 170+00 174+00 400 +15,000 +46,900 052+64 054+00 135 +800 +5,000 174+00 178+00 400 +13,400 +46,400 174+00 182+00 4600 400 +11,700 </td <td>Poi</td> <td>044+25</td> <td>045+07</td> <td>190</td> <td>-700</td> <td>+3,600</td> <td></td> <td>142+00</td> <td>146+00</td> <td>400</td> <td>+15,700</td> <td>+43,600</td> <td>eac</td>	Poi	044+25	045+07	190	-700	+3,600		142+00	146+00	400	+15,700	+43,600	eac
Utility 046+00 046+89 200 -400 +5,900 046+89 049+00 250 +1,300 +12,200 049+00 050+50 100 +900 +5,500 050+50 051+00 100 +1,200 +5,600 051+00 052+64 240 +3,000 +12,600 052+64 054+00 135 +800 +5,000 054+00 056+56 380 +2,800 +14,800 054+00 056+56 380 +2,800 +14,800 174+00 178+00 400 +15,000 +46,300 178+00 182+00 400 +13,400 +40,600 182+00 186+00 400 +13,400 +40,600 182+00 186+00 400 +11,700 +36,500 190+00 190+00 400 +11,700 +36,600 190+00 194+00 400 +5,800 +20,100 202+00 206+00 400 +5,800		045+07	046+00	200	-900	+3,700		146+00	150+00	399	+14,800	+44,700	h
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$			Subtotal	2,285	-10,000	-6,000		150+00	154+00	385	+14,200	+42,400	
$\frac{6}{52+64} = \frac{1}{500} + \frac{1}{100} + $		046+00	046+89	200	-400	+5,900		154+00	158+00	383	+14,300	+43,500	
$\frac{69}{100} = \frac{100}{100} = \frac{100}{100} = \frac{100}{100} = \frac{100}{1100} = \frac{1000}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{100} =$	oin)	046+89	049+00	250	+1,300	+12,200		158+00	162+00	386	+14,700	+44,500	
$\frac{6}{52+64} = \frac{1}{500} + \frac{1}{100} + $	Gr	049+00	050+50	100	+900	+5,500		162+00	166+00	393	+15,700	+50,700	
$\frac{69}{100} = \frac{100}{100} = \frac{100}{100} = \frac{100}{100} = \frac{100}{1100} = \frac{1000}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{1100} = \frac{100}{100} =$	h of	050+50	051+00	100	+1,200	+5,600		166+00	170+00	394	+15,500	+52,200	
Subtotal 1,405 +9,600 +61,600 Note: Elevations are referenced to NGVD 1929. 182+00 186+00 400 +13,400 +40,600 190+00 190+00 400 +11,700 +36,500 190+00 194+00 400 +5,800 +20,100 198+00 202+00 400 +6,900 +29,100 202+00 206+00 400 +9,400 +44,700 206+00 210+00 400 +9,700 +38,800 210+00 214+00 400 +7,100 +30,100		051+00	052+64	240	+3,000	+12,600		170+00	174+00	400	+15,000	+46,900	
Subtotal 1,405 +9,600 +61,600 Note: Elevations are referenced to NGVD 1929. 182+00 186+00 400 +13,400 +40,600 190+00 190+00 400 +11,700 +36,500 190+00 194+00 400 +5,800 +20,100 198+00 202+00 400 +6,900 +29,100 202+00 206+00 400 +9,400 +44,700 206+00 210+00 400 +9,700 +38,800 210+00 214+00 400 +7,100 +30,100	nt (S	052+64	054+00	135	+800	+5,000		174+00	178+00	400	+14,900	+46,300	
Note: Elevations are referenced to NGVD 1929. 186+00 190+00 400 +11,700 +36,500 190+00 194+00 400 +9,100 +34,600 194+00 198+00 400 +5,800 +20,100 198+00 202+00 400 +6,900 +29,100 202+00 206+00 400 +9,400 +44,700 206+00 210+00 400 +9,700 +38,800 210+00 214+00 400 +7,100 +30,100	Poir	054+00	056+56	380	+2,800	+14,800		178+00	182+00	400	+15,000	+46,400	
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$			Subtotal	1,405	+9,600	+61,600		182+00	186+00	400	+13,400	+40,600	
194+00198+00400+5,800+20,100198+00202+00400+6,900+29,100202+00206+00400+9,400+44,700206+00210+00400+9,700+38,800210+00214+00400+7,100+30,100Subtotal16,105+518,900		Note: Elev	ations are re	eferenced	to NGVD	1929.		186+00	190+00	400	+11,700	+36,500	
198+00202+00400+6,900+29,100202+00206+00400+9,400+44,700206+00210+00400+9,700+38,800210+00214+00400+7,100+30,100Subtotal16,105+518,900								190+00	194+00	400	+9,100	+34,600	
202+00206+00400+9,400+44,700206+00210+00400+9,700+38,800210+00214+00400+7,100+30,100Subtotal16,105+518,900+1,431,300								194+00	198+00	400	+5,800	+20,100	1
206+00210+00400+9,700+38,800210+00214+00400+7,100+30,100Subtotal16,105+518,900+1,431,300								198+00	202+00	400	+6,900	+29,100	
206+00210+00400+9,700+38,800210+00214+00400+7,100+30,100Subtotal16,105+518,900+1,431,300								202+00	206+00	400	-	-	
210+00 214+00 400 +7,100 +30,100 Subtotal 16,105 +518,900 +1,431,300										400	-		1
Subtotal 16,105 +518,900 +1,431,300													
												· ·	
Dura 10tal 10tal 12,755 1010,700 11,159,900								Bald He		22,755	+518,700	+1,459,900	

 Table 3.2: Bald Head Island shoreline volume change (November 2020 to May 2021).

	Volume Change					Volume Cha						
	Start Station	End Station	Reach (FT)	Above +2.51 (FT)	Above -16 (FT)		Start Station	End Station	Reach (FT)	Above +2.51 (FT)	Above -16 (FT)	
	Jetty	000+00	160	-500	-3,600		056+56	060+51	423	+5,500	+18,200	
	000+00	004 + 00	400	-400	-5,700		060+51	065+50	510	+9,100	+28,100	
	004+00	008+00	400	+200	-1,800		065+50	069+46	423	+11,000	+31,600	
	008+00	010+00	200	0	-600		069+46	073+39	442	+13,100	+38,300	
ch	010+00	012+00	200	0	-500		073+39	076+37	516	+16,400	+42,200	
Bea	012+00	014+00	200	0	-300		076+37	084+16	611	+17,900	+38,100	
West Beach	014+00	016+00	200	+300	+100		084+16	088+23	471	+14,200	+31,800	
W.	016+00	018+00	200	+500	+900		088+23	092+15	455	+14,200	+33,000	
	018+00	020+00	200	+600	+600		092+15	097+10	536	+15,100	+32,300	
	020+00	024+00	400	-700	-11,100		097+10	102+08	525	+14,700	+31,200	
	024+00	028+00	400	-5,400	-23,300		102+08	106+00	436	+13,200	+25,500	
		Subtotal	2,960	-5,400	-45,300		106+00	110+00	400	+12,300	+21,100	
	028+00	032+00	395	-6,600	-17,800		110+00	114+00	388	+12,700	+23,500	
	032+00	034+00	200	-700	3,400		114+00	118+00	407	+14,200	+26,700	
in)	034+00	036+00	210	1,800	16,000		118+00	122+00	413	+13,200	+27,400	
of Groin)	036+00	038+00	230	2,000	11,200		122+00	126+00	405	+13,100	+27,100	
	038+00	039+60	230	-800	-6,300		126+00	130+00	405	+14,700	+30,700	
Point (North	039+60	041+50	220	-4,000	-17,000		130+00	134+00	398	+14,800	+34,900	\sim
(No	041+50	043+47	220	-4,200	-18,500		134+00	138+00	401	+14,700	+37,300	out
int (043+47	044+25	190	-3,600	-14,500		138+00	142+00	400	+15,400	+38,600	h B
Po	044+25	045+07	190	-4,200	-11,400		142+00	146+00	400	+14,800	+38,800	South Beach
	045+07	046+00	200	-3,400	-3,300		146+00	150+00	399	+14,300	+42,300	
		Subtotal	2,285	-23,700	-58,200		150+00	154+00	385	+14,800	+45,100	
	046+00	046+89	200	-1,000	+2,200		154+00	158+00	383	+14,600	+45,500	
uth of Groin)	046+89	049+00	250	+1,200	+4,000	-	158+00	162+00	386	+15,000	+45,200	
f Gr	049+00	050+50	100	+800	+1,900		162+00	166+00	393	+16,400	+44,200	
th of	050+50	051+00	100	+1,000	+2,500		166+00	170+00	394	+16,200	+46,800	
Sout	051+00	052+64	240	+2,800	+7,800		170+00	174+00	400	+17,400	+50,700	
Point (So	052+64	054+00	135	+1,100	+4,100		174+00	178+00	400	+17,600	+47,500	
Poi	054+00	056+56	380	+3,700	+14,100		178+00	182+00	400	+15,600	+41,800	
		Subtotal	1,405	+9,600	+36,600		182+00	186+00	400	+14,700	+41,000	
Note: Elevations are referenced to NGVD 1929.							186+00	190+00	400	+12,700	+41,700	
							190+00	194+00	400	+8,200	+25,500	
							194+00	198+00	400	+5,500	+15,300	
1							198+00	202+00	400	+6,900	+32,500	
1							202+00	206+00	400	+11,800	+49,000	
1							206+00	210+00	400	+13,700	+43,700	
1							210+00	214+00	400	+7,500	+20,200	
								Subtotal	16,105	+507,200	+1,334,400	
							Bald He	ead Total	22,755	+487,700	+1,267,500	

 Table 3.3: Bald Head Island shoreline volume change (May 2020 to May 2021).

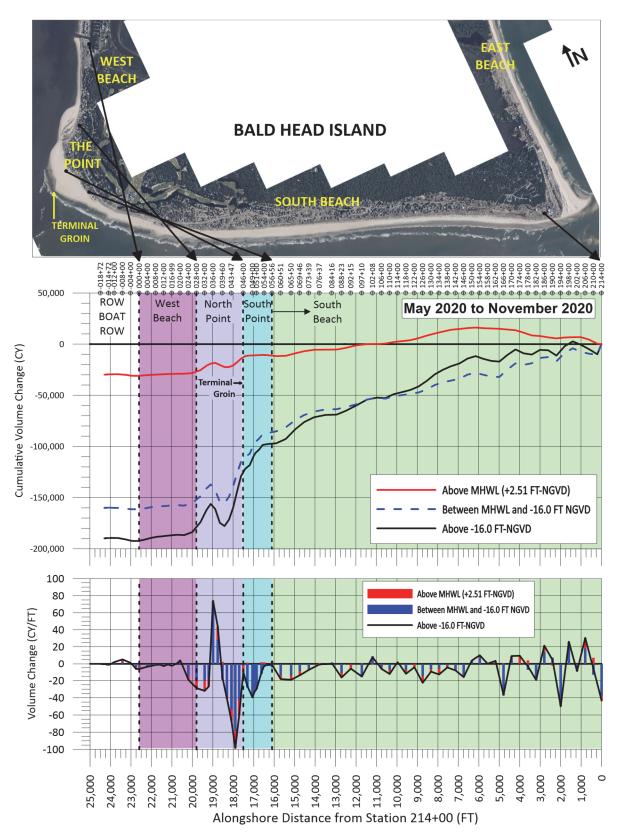


Figure 3.1: Volume change along the Bald Head Island shoreline between May 2020 and November 2020.

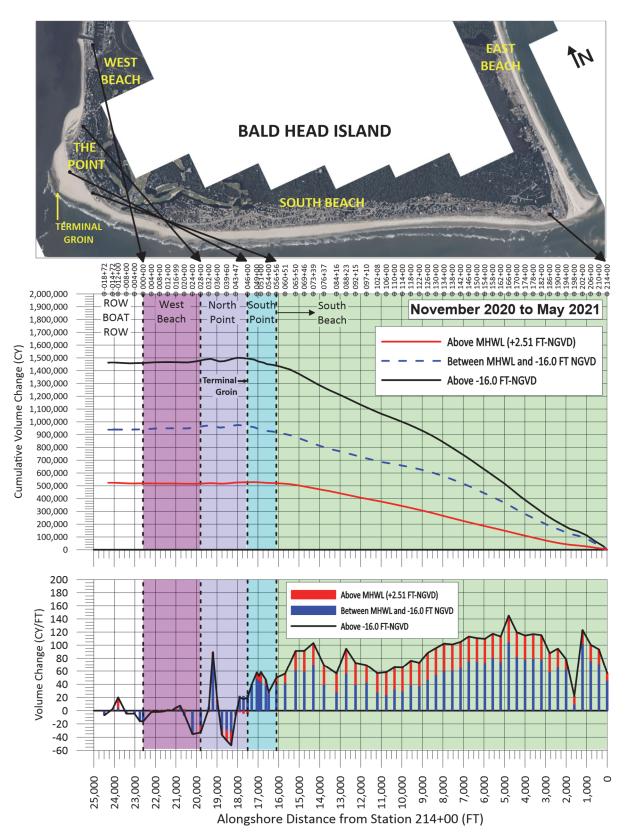


Figure 3.2: Volume change along the Bald Head Island shoreline between November 2020 and May 2021.

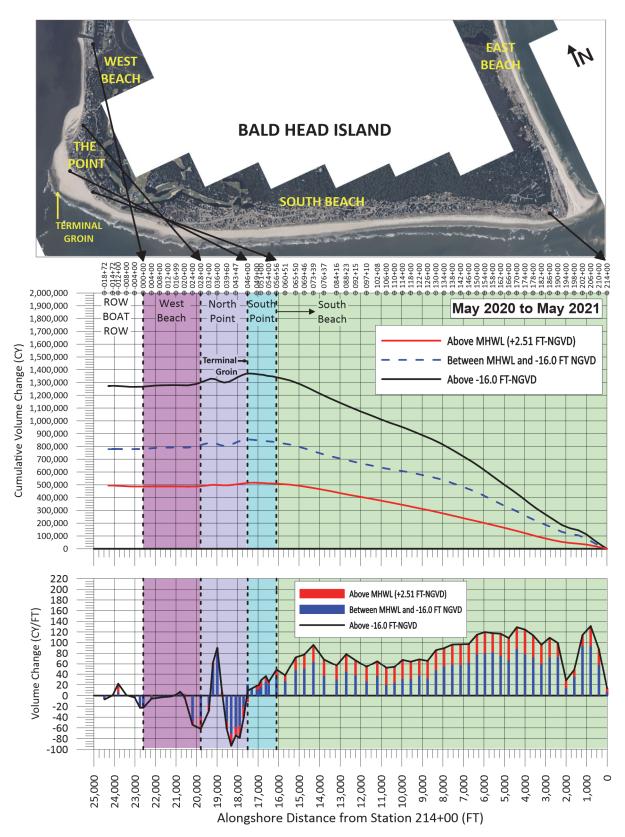


Figure 3.3: Volume change along the Bald Head Island shoreline between May 2020 and May 2021 (Year 20).

		Location Relative to Nov. 2000					Location Relative to Nov. 2000			
	Station	May 2020	Nov. 2020	May 2021		Station	May 2020	Nov. 2020	May 2021	
	000+00	+105.6	+74.5	+68.4		060+51	+47.2	+1.8	+37.9	
	004+00	+21.9	+15.8	+21.1		065+50	+86.0	+52.9	+207.5	
	008+00	+8.7	-1.7	+4.0		069+46	+129.8	+101.0	+249.5	
h	010+00	No Nov	ember 2000) profile		073+39	+175.2	+146.5	+295.9	
eac	012+00	+30.6	+22.0	+31.8		076+37	+189.5	+171.3	+312.6	
t B	014+00	No Nov	ember 2000) profile		084+16	+189.4	+175.8	+299.3	
West Beach	016+00	+37.2	+20.3	+32.2		088+23	+218.3	+180.3	+337.3	
M	018+00	No Nov	ember 2000) profile	-	092+15	+180.6	+162.6	+287.4	
	020+00	+205.5	+125.6	+226.0		097+10	+163.9	+150.4	+255.7	
	024+00	+469.9	+404.5	+372.6		102+08	+139.2	+132.6	+243.8	
	028+00	+368.4	+268.6	+212.8		106+00	+148.8	+130.2	+270.7	
	032+00	+155.4	+61.9	+22.7	South Beach	110+00	+151.0	+146.3	+281.5	
(1	034+00	No Nov	ember 2000) profile		114+00	+157.9	+153.7	+290.6	
roir	036+00	-9.5	-26.7	+125.7		118+00	+186.9	+158.6	+323.1	
f G	038+00	No Nov	ember 2000) profile		122+00	+217.4	+198.7	+359.9	
h o	039+60	+175.0	+99.4	-2.3		126+00	+233.5	+203.7	+400.6	
lort	041+50	No Nov	ember 2000) profile		130+00	+236.8	+216.3	+412.5	
t (N	043+47	+53.2	-103.6	-130.7		134+00	+246.5	+217.2	+416.6	
Point (North of Groin)	044+25	No Nov	ember 2000) profile		138+00	+244.5	+225.1	+416.4	
Р	045+07	+132.4	-28.5	-43.1		142+00	+218.0	+203.3	+396.4	
	046+00	No Nov	ember 2000) profile		146+00	+204.2	+188.6	+377.7	
[]	046+89	+272.7	+256.2	+239.5		150+00	+184.4	+182.6	+367.1	
Point (South of Groin)	049+00	No Nov	ember 2000) profile		154+00	+172.0	+174.4	+354.9	
of C	050+50	No Nov	ember 2000) profile		158+00	+154.7	+157.3	+339.4	
uth	051+00	No Nov	ember 2000) profile		162+00	+137.1	+152.4	+333.7	
(Sol	052+64	+176.8	+139.9	+131.7		166+00	+154.7	+164.5	+359.0	
oint	054+00		ember 2000			170+00	+132.2	+153.2	+344.6	
P	056+56	+90.8	+67.3	+186.8		174+00	+113.7	+149.8	+335.9	
Positi	ve values in	dicate shor	eline adva	nce		178+00	+134.7	+154.8	+344.5	
	ve to the pre					182+00	+142.4	+151.3	+337.9	
-	tive values i		reline eros	ion and		186+00	+110.9	+156.7	+300.8	
are hi	ghlighted in	red.				190+00	+92.0	+98.4	+230.6	
						194+00	+69.0	+72.5	+153.2	
						198+00	+3.6	+36.8	+99.4	
						202+00	-35.3	-18.2	+82.9	
						206+00	-127.1	-90.2	+23.0	
						210+00	-191.6	-156.6	-65.0	
						214+00	-245.4	-275.7	-209.2	

Table 3.4: Location of the **BERM** (+6.0 ft-NGVD) relative to the November 2000 (pre-2001 fill) location for selected monitoring surveys.

		Location	Relative to N	Nov. 2000			Location	Relative to N	lov. 2000
	Station	May 2020	Nov. 2020	May 2021		Station	May 2020	Nov. 2020	May 2021
	000+00	+92.5	+70.3	+59.6		060+51	+36.5	+0.4	+127.9
	004+00	+14.3	+10.8	+8.3		065+50	+79.9	+55.2	+209.0
	008+00	+2.4	-0.3	-0.7		069+46	+118.2	+100.6	+253.4
ų	010+00	No Nove	ember 2000) profile		073+39	+180.8	+153.0	+308.9
eac	012+00	-12.0	-15.9	-12.5		076+37	+199.6	+171.0	+323.6
West Beach	014+00	No Nove	ember 2000) profile		084+16	+200.9	+190.5	+306.5
/est	016+00	+17.4	+9.6	+26.8		088+23	+235.9	+208.5	+353.4
M	018+00	No Nove	ember 2000) profile		092+15	+186.2	+171.6	+295.3
	020+00	+235.4	+234.4	+239.1		097+10	+165.5	+147.7	+262.9
	024+00	+469.2	+399.5	+355.4		102+08	+135.5	+131.1	+249.1
	028+00	+268.5	+175.4	+120.8		106+00	+162.4	+141.1	+274.3
	032+00	+63.6	-26.6	+1.4		110+00	+173.2	+145.7	+277.6
	034+00	No Nove	ember 2000) profile		114+00	+182.1	+152.1	+304.0
nio	036+00	-49.1	+102.2	+115.8		118+00	+185.8	+158.1	+340.0
Gr	038+00	No Nove	ember 2000) profile		122+00	+218.5	+190.1	+375.4
h of	039+60	+115.0	+45.6	-55.2		126+00	+239.0	+190.8	+402.5
ort	041+50	No Nove	ember 2000) profile	_	130+00	+233.3	+206.8	+416.9
t (N	043+47	-54.2	-202.3	-223.3	acł	134+00	+246.0	+210.8	+423.4
Point (North of Groin)	044+25	No Nove	ember 2000) profile	Be	138+00	+234.9	+206.7	+407.7
Р	045+07	+122.2	-23.3	-73.7	th	142+00	+205.0	+191.1	+391.1
	046+00	No Nove	ember 2000) profile	South Beach	146+00	+195.2	+170.2	+375.8
(046+89	+276.6	+287.1	+306.2		150+00	+178.1	+170.4	+365.6
Point (South of Groin)	049+00	No Nove	ember 2000) profile		154+00	+170.6	+163.0	+354.9
of G	050+50	No Nove	ember 2000) profile		158+00	+161.7	+155.7	+346.0
lth	051+00	No Nove	ember 2000) profile		162+00	+156.0	+146.4	+340.1
(Sot	052+64	+161.4	+133.8	+240.8		166+00	+148.5	+142.9	+339.1
int	054+00	No Nove	ember 2000) profile		170+00	+118.2	+135.0	+331.9
Po	056+56	+71.5	+56.5	+190.2		174+00	+124.4	+142.5	+339.8
Positi	ve values in	dicate shor	eline adva	nce		178+00	+146.0	+159.6	+357.8
	ve to the pre					182+00	+138.8	+141.2	+339.2
-	tive values i		reline eros	ion and		186+00	+116.3	+134.1	+299.8
are hi	are highlighted in red.					190+00	+81.6	+77.2	+224.2
						194+00	+49.8	+50.4	+142.0
						198+00	-7.3	+34.1	+107.7
					202+00	-60.0	-37.5	+96.8	
						206+00	-148.6	-107.9	+33.9
						210+00	-204.8	-177.0	-67.2
						214+00	-265.5	-282.0	-213.4

Table 3.5: Location of the <u>MHWL</u> (+2.51 ft-NGVD) relative to the November 2000 (pre-2001fill) location for selected monitoring surveys.

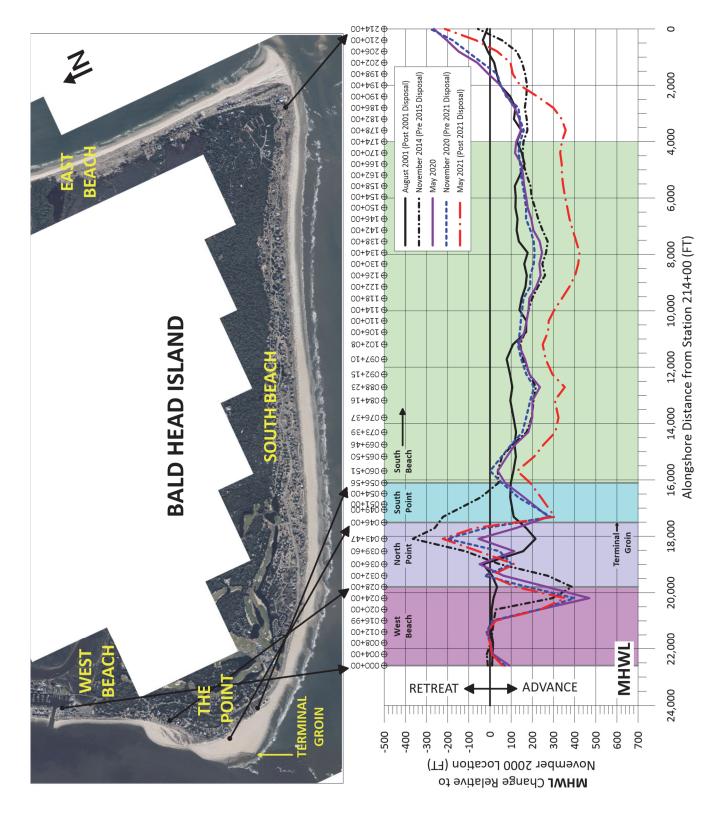


Figure 3.4: Location of the MHWL (+2.51 ft-NGVD) relative to the November 2000 (pre-2001 fill) location.

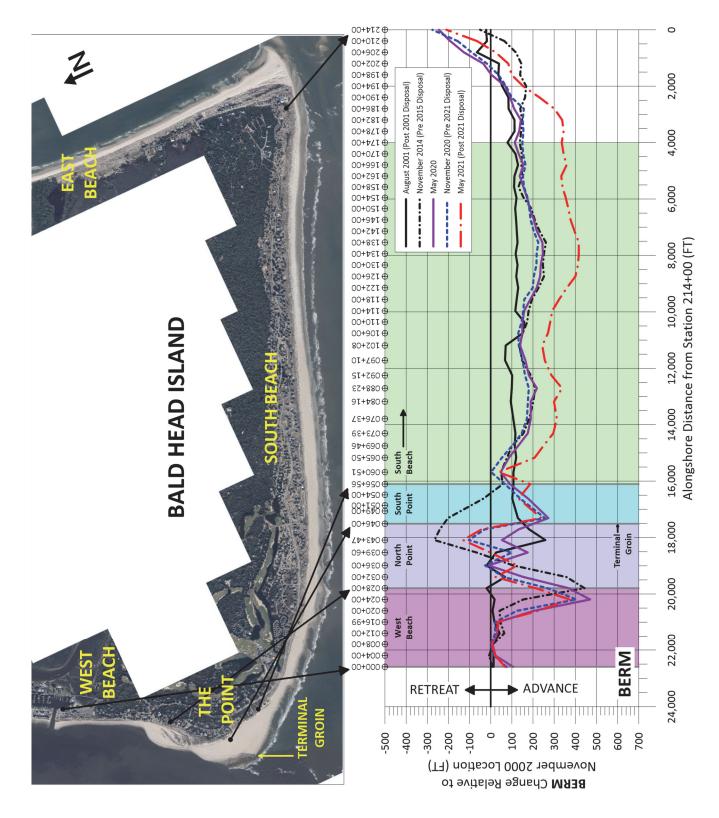


Figure 3.5: Location of the BERM (+6.00 ft-NGVD) relative to the November 2000 (pre-2001 fill) location.

In the net, <u>West Beach</u> was relatively moderately erosion during this period losing -5,600 cy above the MHWL and -12,700 cy between the MHWL and the -16 ft-NGVD contour. Overall West Beach lost roughly -18,300 cy above the -16 ft contour. The largest loss occurred closest to the Point northernmost 1,600 ft (Jetty to Sta 016+00). During this period, the berm receded by an average of -33 ft while the MHWL receded by -20 ft.

The entire 3,690 ft of <u>"the Point"</u> shoreline (Sta. 28+00 to 56+56) was net erosional during this monitoring period, losing -77,200 cy above -16 ft-NGVD. For purposes of evaluating the impacts of the terminal groin completed in November 2015, "the Point" shoreline is subdivided into two reaches with Sta. 46+00, the approximate location of the terminal groin, as the dividing station. North of the terminal groin (Sta. 28+00 to 46+00), the shoreline lost -13,700 cy above the MHWL and -52,500 cy above -16 ft-NGVD. Along this reach, the berm receded by roughly -81 ft on average and the MHWL by -75 ft. South of the terminal groin (Sta. 46+00 to 56+56), the shoreline was volumetrically stable above the MHWL (on average) and lost -25,000 cy above the -16 ft-NGVD contour adjacent to the channel. The berm and MHWL receded by averages of -46 ft and -32 ft, respectively.

<u>South Beach</u> was net erosional during the period, losing roughly -11,700 cy above the MHWL and -96,900 cy above -16 ft-NGVD. All but 10 (28 of 38) of the monitoring stations were net erosional above -16 ft-NGVD. During this period, the berm receded by an average of -5 ft while the MHWL receded by -10 ft.

3.3.2 Survey Period: November 2020 to May 2021 (Post-Federal Beach Fill)

As depicted in **Figure 3.2** and **Table 3.2**, the island-wide *net* volume change was a large-scale gain of approximately +1.46 Mcy above -16 ft-NGVD. Consistent with the overall fill volume placed, the berm and MHWL were extended seaward along the entirety of the South Beach shoreline.

In the net, <u>West Beach</u> was relatively stable during this period with a negligible gain of above the MHWL. Overall West Beach lost roughly -27,000 cy above the -16 ft contour. During this period, the berm advanced by an average of +7 ft while the MHWL receded by -5 ft.

Along <u>"the Point"</u> shoreline north of the terminal groin, the beach lost -10,000 cy above the MHWL and gained +4,000 cy above the -16 ft-NGVD contour. During this period, the berm retreated by an average of -23 ft and the MHWL retreated by an average of -3 ft. Along "the Point" shoreline south of the terminal groin, the beach gained +61,600 cy above -16 ft-NGVD. During this period, the berm advanced by an average of +11 ft and the MHWL by +82 ft.

<u>South Beach</u> was net accretional during the period due to the federal beach fill, gaining roughly -+1.43 Mcy above the -16 ft-NGVD. Above the MHWL, the shoreline gained 518,900 cy. During this period the berm and MHWL advanced by averages of roughly +148 ft and +160 ft, respectively, due to alongshore fill placement.

3.3.3 Year 19 Monitoring Results: May 2020 to May 2021 – Post Fill (Excluding East Beach & Row Boat Row)

During Year 20 in its entirety, the island experienced a net gain of +1.267 Mcy above the -16 ft contour (see **Table 3.3**). Approximately 70 percent of this gain occurred during the 2nd six months of the monitoring period (Nov 2020 to May 2021), due to the federal beach disposal project at South Beach.

Along West Beach, the shoreline lost approximately -5,400 cy above the MHWL and -45,300 cy above -16 ft-NGVD. During Year 20, the berm receded by an average of -26 ft and the MHWL by -25 ft.

The entire Point shoreline (north and south of the terminal groin), experienced a net loss of roughly -14,100 cy above the MHWL and -21,600 cy above -16 ft-NGVD. During this period, the berm receded by an average of -71 ft and the MHWL receded by -21 ft.

The South Beach shorefront gained +507,200 cy above the MHWL and +1.334 Mcy above -16 ft-NGVD. During this period, the recently improved fill berm and MHWL advanced varying amounts (but in a non-equilibrated configuration).

3.3.4 Long-Term Beach Changes: November 2000 to May 2020 (Post Federal Disposal)

For purposes of tracking gross sand placement performance, **Figure 3.6** plots a time history of cumulative volume change relative to November 2000 conditions. **Figure 3.7** presents net volumetric change (alongshore above -16 ft NGVD) for the maximum period of comparison to date (*i.e.* November 2000 and May 2020). In both figures the effects of direct sand placement over time are included. As with other similar analyses over the last decade, East Beach, Cape Fear and Row Boat Row are *excluded* from this analysis.

The classic "saw-tooth" effects of episodic sand placement, as reflected in **Figure 3.6**, are indicative of the periodic infusion of sand along South Beach at Bald Head Island associated with the placement of sand during initial construction of the channel deepening project, five (5) subsequent beach disposal operations pursuant to the WHSMP, the proactive beach renourishment project constructed by the Village in 2009/10 and to a smaller degree the emergency fill of 2012. The Village 1.85 Mcy fill was constructed with the knowledge gained through monitoring that certain irreparable large-scale impacts to Bald Head Island would predictably occur as a direct result of the proposed diversion of channel maintenance material in 2009 to Oak Island. *Note – a similar diversion of Federal sand occurred in the summer of 2018*. As a result of the 2018 federal sand disposal at Oak Island, the Village constructed a 1.1 Mcy interim beach fill at South Beach in the fall/winter of 2018/19. The most recent federal beach disposal project was performed in the early months of 2021 (Jan – April) along South Beach on Bald Head Island.

Table 3.6 presents a chronology of sediment volumes (measured in-place) for the three (3) segments of shoreline noted between the benchmark survey of November 2000 and present (*i.e.* May 2021). Currently, within the **approximate** 22,755 ft of shoreline considered, there is a net gain of +3,747,000 cy. However, after removing the effects of the sand artificially placed along the Bald Head Island shoreline since the 2000 deepening project, the net change in Island-wide volume (exclusive of East Beach and the Cape Fear Point) is a measured sediment *loss* of -8,035,800 cy. It is important to note that the chronology of sand volumes presented by this Table reflects the *actual volumes* of sand *measured in-place* by survey and therefore is not related to projections based upon *estimated* volumes dredged in the channel or borrow site, *estimated* sand volumes placed, contractual "net pay" volumes, etc.

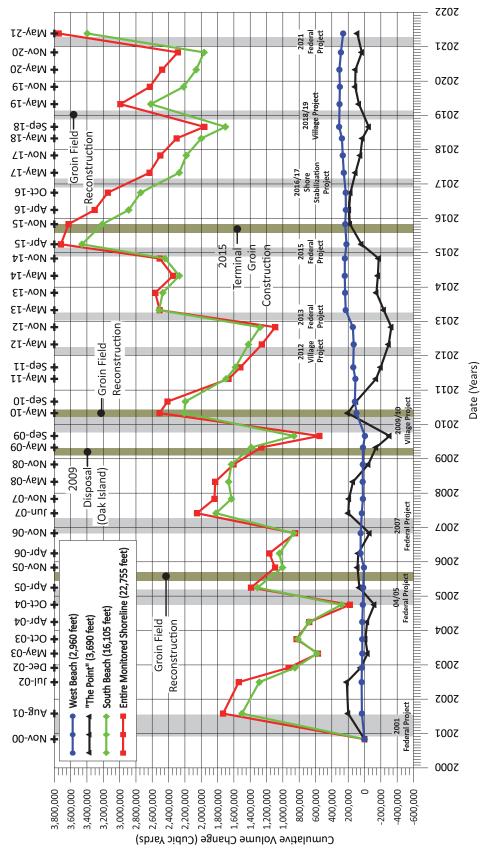


Figure 3.6: Cumulative volume change (above -16 ft-NGVD) relative to November 2000 conditions.

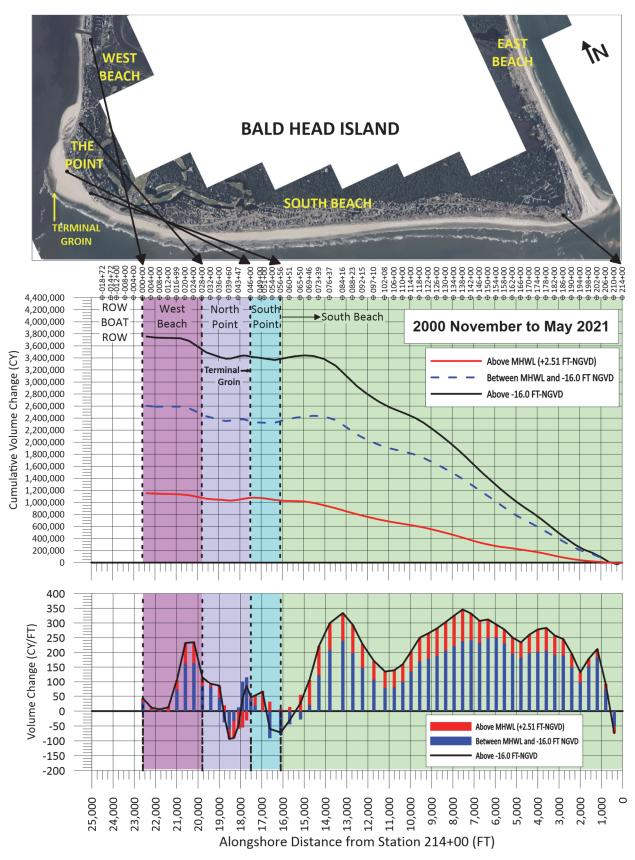


Figure 3.7: Volume change along the Bald Head Island shoreline between November 2000 and May 2021.

				Volume Change Above -16 ft-NGVD (CY)				
	Start	End	Span	West	The			
Period	Date	Date	(Months)	Beach	Point	South Beach	Total	
Construction ¹	Nov. 2000	Aug. 2001	9	+31,900	+199,500	+1,501,800	+1,733,200	
Year 1	Aug. 2001	Jul. 2002	11	+2,900	+17,400	-213,300	-193,000	
Year 2	Jul. 2002	May 2003	10	-8,000	-255,500	-707,400	-970,900	
Year 3	May 2003	Apr. 2004	11	+1,000	+6,500	+99,900	+107,400	
Year 4 (2004/05 Project) ²	Apr. 2004	Apr. 2005	12	-11,800	+94,700	+631,200	+714,100	
Year 5 (2006 WB Project) ³	Apr. 2005	Apr. 2006	12	+32,000	+13,300	-270,200	-224,900	
Year 6 (2007 Project) ⁴	Apr. 2006	Jun. 2007	14	-15,400	+123,500	+778,100	+886,200	
Year 7	Jun. 2007	May 2008	11	-10,300	-58,200	-154,600	-223,100	
Year 8	May 2008	May 2009	12	-3,400	-282,800	-278,200	-564,400	
Year 9 (2009/10 Project) ⁵	May 2009	May 2010	12	+79,300	+346,000	+821,300	+1,246,600	
Year 10	May 2010	May 2011	12	+13,200	-346,100	-512,700	-845,600	
Year 11 (2012 Beach Fill) ⁶	May 2011	May 2012	12	+20,800	-154,600	-273,300	-407,100	
Year 12 (2013 Disposal) Project) ⁷	May 2012	May 2013	12	+97,600	+59,800	+1,093,900	+1,251,300	
Year 13	May 2013	May 2014	12	+11,600	+72,100	-247,500	-163,800	
Year 14 (2015 Disposal) Project) ⁸	May 2014	April 2015	11	-20,400	+201,800	+1,191,800	+1,373,200	
Year 15	April 2015	April 2016	12	+7,200	+151,800	-572,500	-413,500	
Year 16	April 2016	May 2017	13	+25,500	-79,000	-619,000	-672,500	
Year 17	May 2017	May 2018	12	+23,200	-84,600	-270,500	-331,900	
Year 18 (2018/19 Beach Fill) ¹⁰	May 2018	May 2019	12	+29,000	+42,200	+619,500	+690,700	
Year 19	May 2019	May 2020	12	+1,200	+42,200	-555,900	-512,500	
Year 20	May 2020	May 2021	12	-45,300	-21,600	+1,334,400	+1,267,500	
Pre-2000 Construction to Year 20	Nov. 2000	May 2021	246	+261,800	+88,400	+3,396,800	+3,747,000	
Pre-2000 Construction to Year 20 (11,699,800 CY of Fill Removed)	Nov. 2000	May 2021	246	NA	NA	NA	-8,035,800	

Table 3.6: Bald Head Island historic net volume change above -16 ft-NGVD (presumed closure depth).

 1 2001 Initial Disposal (1,849,500± CY); 2 2005 Beach Disposal (1,217,000± CY); 3 2006 West Beach Fill (47,800± CY); 4 2007 Beach Disposal (978,500± CY); 5 2009/10 Beach Fill (1,850,000± CY); 6 2012 Beach Fill (138,000± CY); 6 2017 Beach Fill (1,850,000± CY); 6 2017 Beach

⁷ 2013 Beach Disposal Fill (1,658,000± CY); ⁸ 2015 Beach Disposal (1,320,000± CY);
 ⁹ 2016/17 Beach Disposal (24,000± CY); ¹⁰ 2018/19 Beach Fill (1,100,000± CY); ¹¹ 2021 Beach Disposal (1,601,000± CY)

The estimated *average* annual loss of sand from the monitored section of Bald Head Island shorefront (excluding East Beach and Row Boat Row) since November 2000, is approximately 401,800 cy per year including the impacts of numerous major storm events. The assignment of an "average" annual long-term rate of sand loss at Bald Head Island however, is *not* necessarily a meaningful indicator of navigation project impact. Such an "average rate" is temporally biased by periods of beach fill equilibration, groin field effectiveness, the occurrence of episodic destabilizing dredging events in close proximity to the island, as well as other physiographic phenomena temporally affecting annualized quantities of alongshore sediment transport – from Bald Head Island – to the navigation channel, -- including meteorological effects – such has Hurricanes Florence, Dorian and Isaias.

3.3.5 MHWL Shoreline Position

As part of the permit required monitoring for the terminal groin project completed in late 2015, the MHWL was surveyed in December 2015 (post-construction), April 2016 (5 months post-construction), June 2017 (19 months post-construction), May 2018 (30 months post-construction), September 2018 (post-Florence), May 2019 (post-fill), November 2019 (post-Dorian), May 2020, November 2020 and May 2021. The selected surveys are plotted in **Figure 3.8**. The purpose of the surveys is to be able to intercompare and assess both updrift fillet conditions and the location of the downdrift shoreline fronting the Cape Fear River. Through May 2021, terminal groin project performance – based upon monitoring – has been both as intended and as predicted.

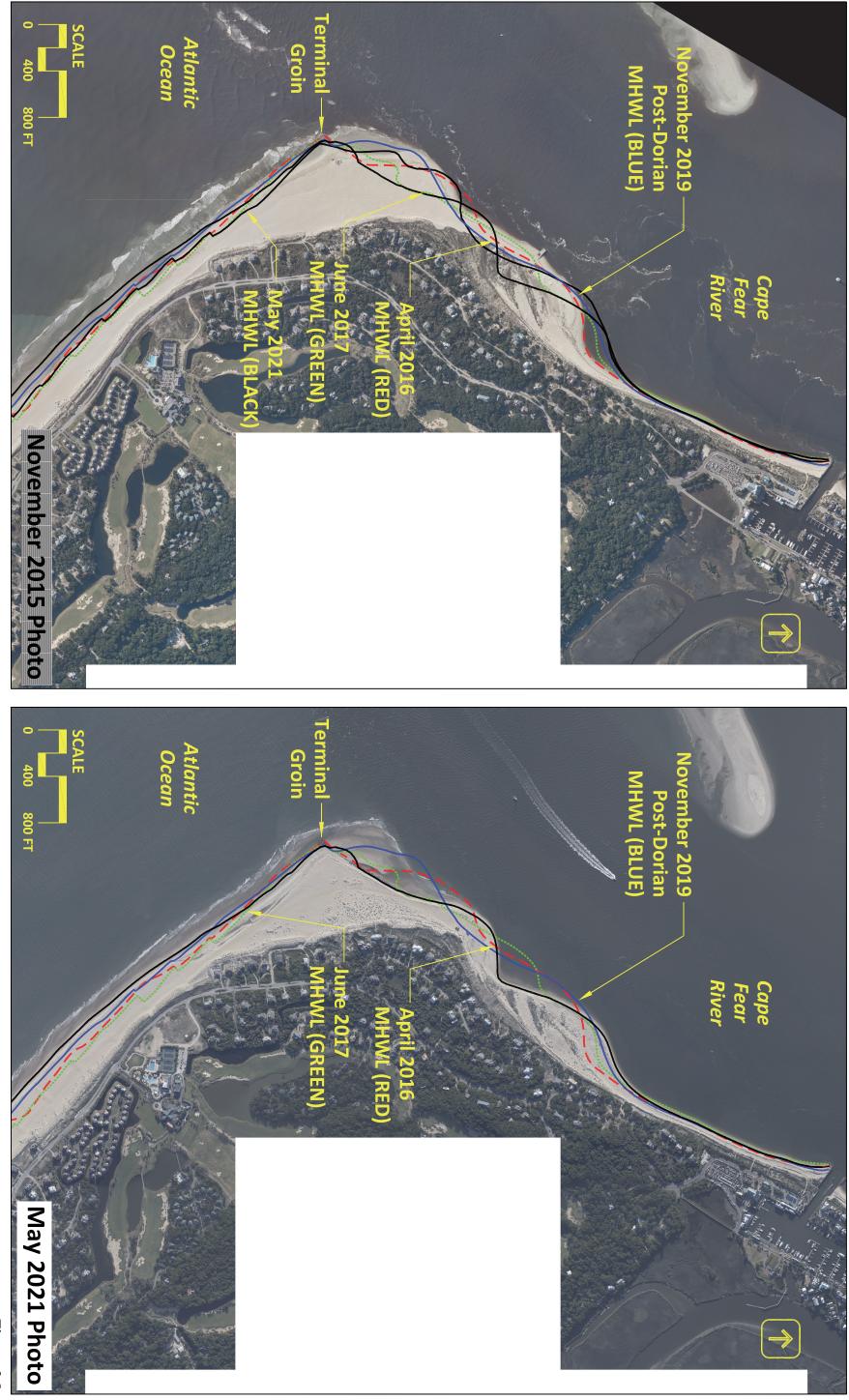


Figure 3.8: MHWL positions in the vicinity of the terminal groin Bald Head Island, NC

3.3.6 Chronology of the Point

Since the construction of the Wilmington Harbor Channel Deepening Project – in about 2001, the spatial configuration of the spit feature (known as the "Point") located at the juncture of South Beach and the entrance channel, has been a focal point of the Village's monitoring program. Accordingly, the chronology of the Point's condition and evolution over time is indicative of the dynamic interaction between the ever increasing rate of sand transport westward along South Beach and the man-altered inlet hydrodynamics, as well as episodic dredging operations which result in sand removal from the island's littoral system. In its simplest sense, the Point has historically been to a large degree, a visual indictor of the processes involved and a potential "bellwether" as to direct and indirect impacts associated with the Navigation Project – irrespective of proactive or remedial actions specified within the Wilmington Harbor Sand Management Plan. The latter take the form of alongshore sand placement events intended to mitigate adverse impacts associated with both project construction in 2000 and episodic channel maintenance required to ensure navigability.

Appendix E includes a high resolution visual chronology of the Point from 1998 to May 2021. Demarcated on each photo panel are the approximate September 2001 (blue line) and May 2021 (red line) apparent vegetation lines. Also placed on each photo are two reference marks (green dots). The variation in spit configuration from the before navigation improvement project photos (1998 and 1999) throughout the last approximate nineteen years for pre- and post-fill timeframes can be easily visualized. Similarly, the advance and recession of the Point, as well as its consistent *net northerly migration* are self-evident. An additional perspective can be gained by an assessment of the locations of the pre-project and present day "vegetation lines" over the 1998 through 2021 timeframe. As had been concluded throughout the numerous years of comprehensive beach monitoring funded by the Village of Bald Head Island – improved conditions along the westernmost segment of South Beach and the Point were documented to last only about 2 years after each federal disposal event – *prior* to terminal groin construction in 2015.

Both long term monitoring, as well as numerical modeling of the Cape Fear River Entrance by Olsen Associates, Inc. (Olsen 2013a), and the abutting Bald Head Island shoreline, indicated that additional structural measures were warranted. As the westernmost segment of South Beach shoreline had "rolled back," the annualized rate of littoral transport at that location had correspondingly increased. Hence, in 2012 the Village initiated the permitting for a 1,300 ft terminal structure intended to both reorient the effective updrift shoreline alignment (so as to reduce annual sediment losses) and to allow for the reconstruction of a protective beach where one now could not be reliably established through sand placement alone. That project was constructed during the summer of 2015. Subsequently, monitoring reports now document a "new dynamic" predicted to result from the implementation of the terminal groin structure. Analytical predictions of shoreline change to both the updrift and downdrift shorelines abutting the structure – via DELFT 3D modeling – were discussed in a detailed report formulated for purposes of both design and permitting of the terminal groin (Olsen 2013a). Additional monitoring data required by Permit are intended to assist in the quantification of the terminal groin effects on littoral processes and resultant shoreline reconfiguration. These include additional transects in the vicinity of the structure as well as an approximate MHWL delineation performed by survey every 6-months.

For the May 2020 to May 2021 monitoring period, the inlet facing shoreline adjacent to the terminal groin continues to realign (as predicted) and adjust to a new equilibrium condition. An intertidal spit formation continues to form on the inlet side of the structure as a result of sediment transported from South Beach through or across the structure. Updrift thereof, portions of the historical Point continue to migrate northward as they did prior to terminal groin construction. This is best represented by the surveyed MHWL locations depicted in **Figure 3.8**. The configuration of the sand fillet updrift of the terminal groin continues to be influenced by the sand tube groin field as fill berms recede and the formerly buried groins become "activated". The most recent Jan – April 2021 beach disposal project placed some 1.61 Mcy along South Beach. The mobilization of a portion of that fill westward will significantly affect the volume of material directed toward, over and through the terminal structure at the Point. Resultant increased rates of sand deposition on the western side of the structure will heavily influence the shoreline configuration at that location during the next 12 months, mol.

3.4 East Beach Shoreline Conditions

In November 2008, East Beach was added to the island-wide beach monitoring program¹⁰. Profiles along the East Beach shoreline are collected at seven (7) monitoring stations starting just north of Cape Fear and extending approximately 6,000 feet northward along the Onslow Bay facing shoreline (see **Figure 2.1**). Plots of these profiles are provided at the end of **Appendix A** (**Figures A-70** to **A-76**). **Tables 3.7** and **3.8** summarize the shoreline and volume changes measured during the May 2020 to November 2020 to May 2021 monitoring periods. **Figure 3.9** depicts the May 2020, November 2020 and May 2021 aerial photographs along southern East Beach.

During the May 2020 to November 2020 period, the East Beach shoreline lost approximately -19,200 cy (-3.2 cy/ft) above the MHWL and gained +34,700 cy (+5.8 cy/ft) below the MHWL for a net total change above -16 ft-NGVD of +15,500 cy (+2.6 cy/ft). During this same period the backshore berm (at elevation +6 ft-NGVD) receded by a spatially weighted average¹¹ of -8.6 ft while the MHWL receded by an average of -4.1 ft.

During the November 2020 to May 2021 winter period, the East Beach shoreline gained approximately +11,400 cy (+1.9 cy/ft) above the MHWL and -27,300 cy (-4.6 cy/ft) below the MHWL for a net total change above -16 ft-NGVD of -15,900 cy (-2.7 cy/ft). During this same period the berm advanced by a spatially weighted average of -15.4 ft while the MHWL advanced by an average of -11.1 ft.

Table 3.9 summarizes the volume changes measured over the entire period of survey record (November 2008 – May 2021). Over the 151-month period, the East Beach shoreline gained approximately +76,500 cy above the MHWL and +350,100 cy above the -16 ft-NGVD contour. Likewise, the backshore berm and MHWL advanced by weighted averages of +36.1 ft and +37.9 ft, respectively over this period. Note – based upon post beach disposal visual observations of East Beach, these long-term volumetric gains are expected to increase substantially over the next 12 months.

¹⁰ Profiles were not acquired at East Beach in the fall of 2009

¹¹ Due to the irregular spacing of the profile stations, the average shoreline changes are weighted based upon the distance between stations and calculated using an average end method.

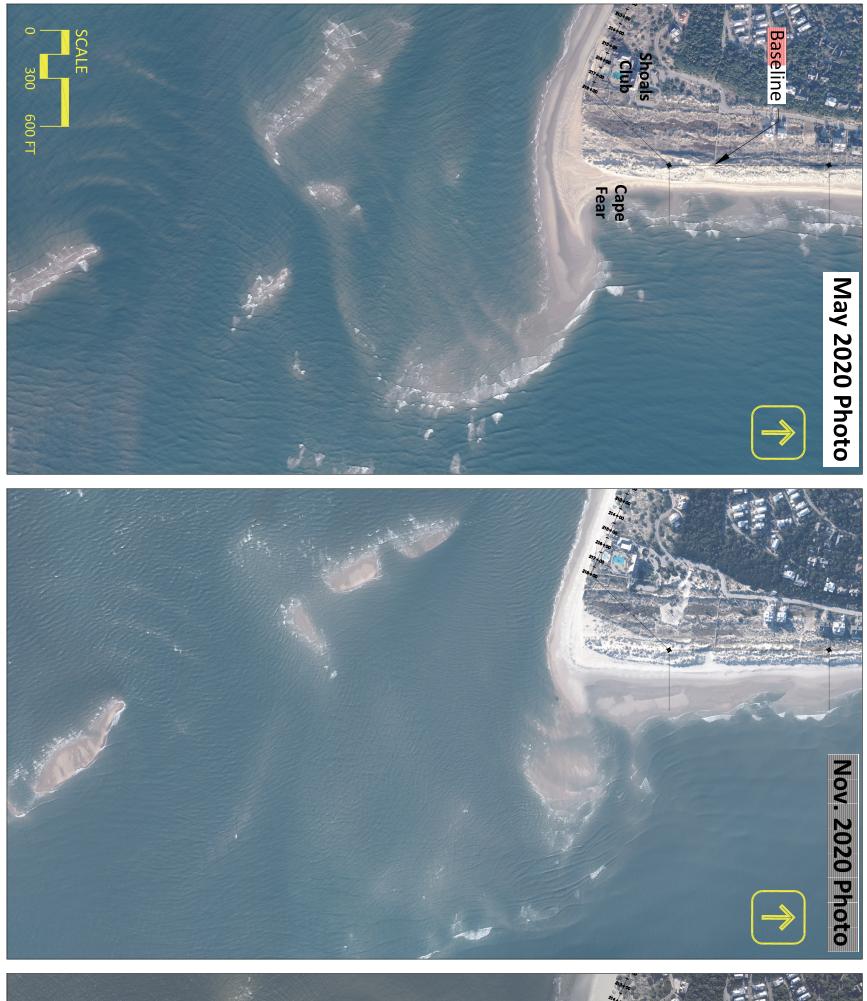
		Volume Change (CY)		Shoreline (Change (FT)	
Station	Reach (FT)	Above MHWL (+2.51 FT)	Above -16 FT	Berm (+6 FT)	MHWL (+2.51 FT)	
224+80				+51.1	+54.7	
	1,000	-6,300	+22,500			
234+80				-37.0	-32.7	
	1,000	-10,000	-23,300			
244+80				-44.8	-32.6	
	1,000	-1,000	+800			
254+80				-10.2	-8.9	
	1,000	-800	+9,700			
264+80				-18.9	-20.6	
	1,000	-600	+700			
274+80				+3.2	+8.3	
	1,000	-500	+5,100			
284+80				-3.9	+2.9	
Total	6,000	-19,200	+15,500	-8.6 (AVG)	-4.1 (AVG)	

Table 3.7: East Beach shoreline and volume changes between May 2020 and November 2020.

Table 3.8: East Beach shoreline and volume changes between November 2020 and May 2021.

		Volume Change			Shoreline	Change (FT)
		Above				
	Reach	MHWL	Above		Berm	MHWL
Station	(FT)	(+2.51 FT)	-16 FT		(+6 FT)	(+2.51 FT)
224+80					+87.9	+104.1
	1,000	+6,300	+32,700			
234+80					+27.6	+19.0
	1,000	-1,200	-20,500			
244+80					-21.3	-32.7
	1,000	-2,500	-27,900			
254+80					-3.3	-8.3
	1,000	+3,400	-600			
264+80					+22.3	+11.3
	1,000	+4,300	+7,800			
274+80					-6.1	-7.6
	1,000	+1,100	-7,400			
284+80					+0.7	-8.3
				1		
Total	6,000	+11,400	-15,900		+15.4 (AVG)	+11.1 (AVG)

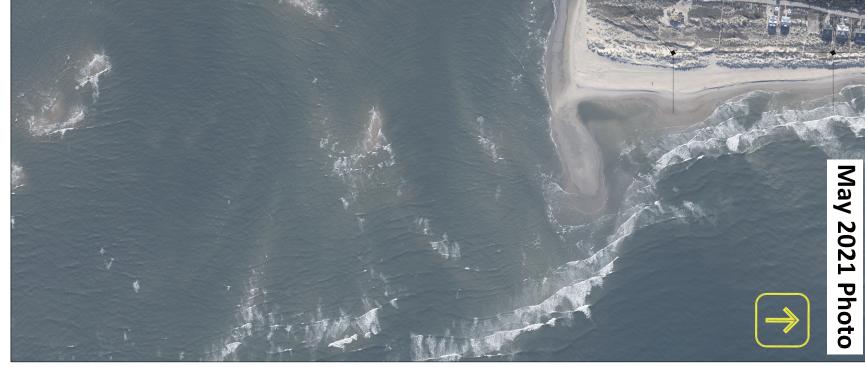
	Volume Change Above Datum (CY)						
Survey Period	Above MHWL (+2.51 ft-NGVD)	Below MHWL to -16 ft-NGVD	Total Change Above -16 ft-NGVD				
November 2008 to May 2009	+700	-65,600	-64,900				
May 2009 to May 2010	-23,300	-8,600	-31,900				
May 2010 to May 2011	+10,600	+18,000	+28,600				
May 2011 to May 2012	+5,700	+87,700	+93,400				
May 2012 to May 2013	+20,000	-41,600	-21,600				
May 2013 to May 2014	+17,700	+105,200	+122,900				
May 2014 to April 2015	-900	+44,100	+43,200				
April 2015 to April 2016	+20,800	-400	+20,400				
April 2016 to May 2017	+4,500	+38,200	+42,700				
May 2017 to May 2018	+31,400	+25,000	+56,400				
May 2018 to May 2019	+9,600	+140,300	+149,900				
May 2019 to May 2020	-12,500	-76,100	-88,600				
May 2020 to May 2021	-7,800	+7,400	-400				
November 2008 to May 2021	+76,500	+273,600	+350,100				





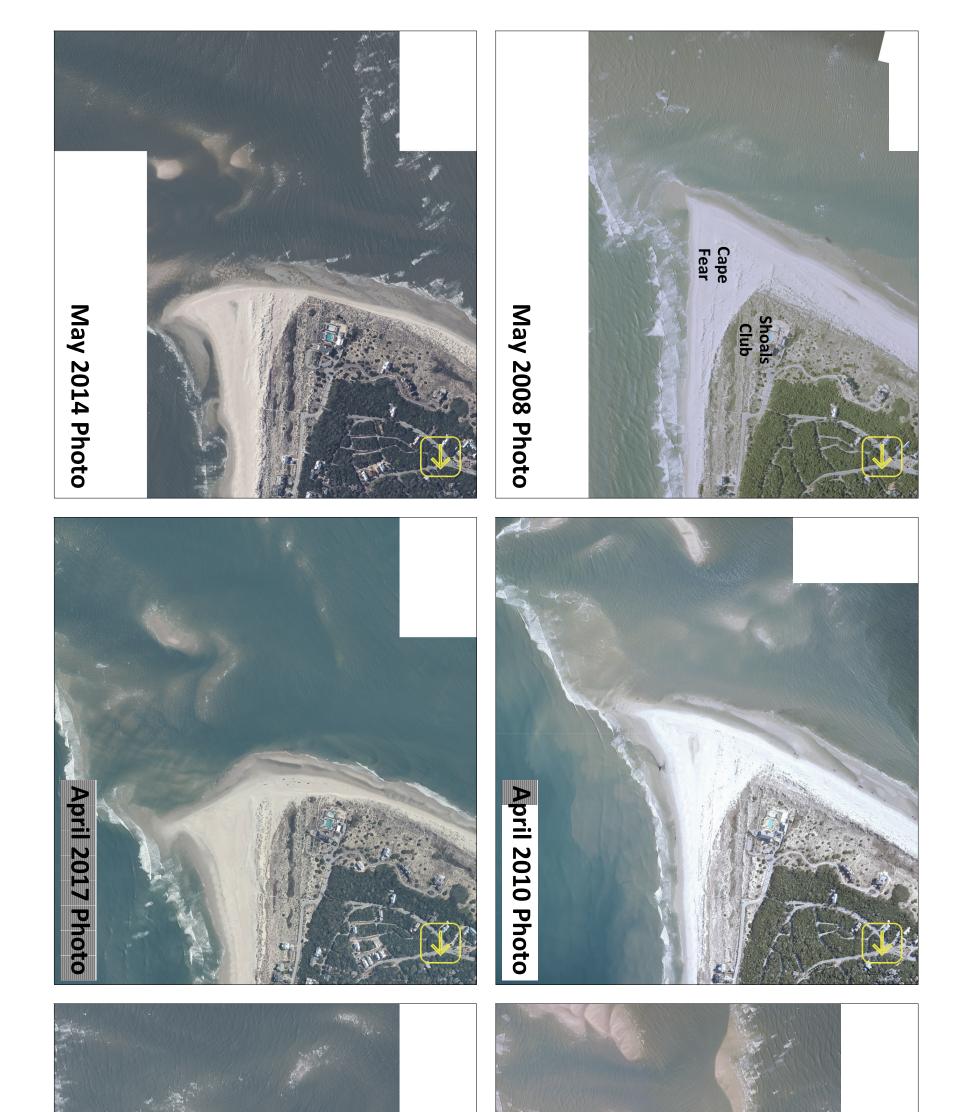
July 2021

Figure 3.9: Cape Fear aerial photography Bald Head Island, NC



As demonstrated by the survey and photographic data (**Figure 3.9**), it can be reasonably assumed that the condition of East Beach at any one time is, has been and will continue to be highly influenced by the configuration of the depositional spit and shoals associated with the "Cape Fear Point". Of further interest are the variations in spit size and orientation over the 12 years (2008-2021) which are depicted by **Figure 3.10**. In its simplest sense, the Cape Fear spit is a highly dynamic feature which is influenced by sand supply originating from both the west (along South Beach) and the north (along East Beach). The Point is also highly susceptible to storm waves originating from *both* the west (Atlantic Ocean) and the east (Onslow Bay) and resultant tidal channels which episodically break through and subsequently influence localized patterns of sand deposition (or erosion).

Although the near-term locations of the Cape Fear spit have been beneficial to East Beach properties, it has typically caused significant shoreline and dune recession seaward of the South Beach Shoals Club facility. That section of shorefront is monitored via beach profiles B-54 and B-55 (Sta. 214+00 and 218+00). The Shoals Club lies approximately mid-way between these two survey stations. May 2021 shoreline conditions (post-beach disposal) are visually shown by Figure B-9 (Appendix B – May 2021 Aerial Photography – page B-10). Between November 2014 and May 2020, the MHWL at B-54 had receded by about -208 ft which equates to a rate of -38 ft/yr. At B-55, over the same period of time, the MHWL had receded -130 ft, or about -24 ft/yr. In contrast, Figure B.9 qualitatively shows improved beach conditions at the two (2) profiles lines noted above due to the most recent federal beach disposal project completed in April 2021. For example, at B-54 the effective MHWL was extended seaward approximately 75 ft between November 2020 and May 2021. At B-55, the MHWL was moved seaward approximately 40 ft. Neither of these conditions represents an "equilibrated" beach profile and is subject therefore to change. Another influencing factor will be whether beach fill placed westward of the Shoals Club facility will eventually move eastward toward Cape Fear, thereby reversing erosional trends experienced over the last two decades, subsequent to the 2001 federal disposal project which benefited the entirety of the South Beach shoreline.



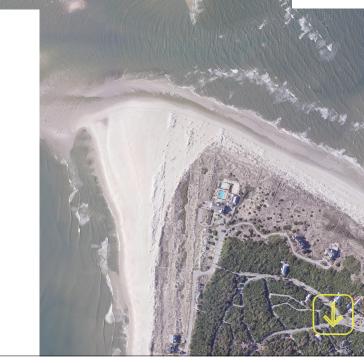
July 2021

48

olsen associates, inc.

Figure 3.10: Cape Fear aerial photography Bald Head Island, NC





3.5 Row Boat Row Shoreline Conditions

In November 2015, the "Row Boat Row" shoreline was added to the island-wide beach monitoring program. Survey data are collected at five (5) monitoring stations starting just north of the marina entrance and extending approximately 1,500 feet northward along the Cape Fear River facing shoreline (see Figure 2.1). Plots of these profiles are provided at the beginning of **Appendix A** (Figures A-1 to A-5). Tables 3.10 and 3.11 summarize the shoreline and volume changes measured during the May 2019 to November 2020 to May 2021 monitoring period (12 months).

In early 2017, after completion of a 26,000 cy beach fill placed by Marcol Dredging along the Row Boat Row shoreline, two detached rock breakwaters were constructed by Intra Coastal Marine Construction. Final acceptance of the project occurred in July 2017. Subsequently, the shorefront within the influence of the two shore parallel structures has equilibrated into a series of discrete crenulate features between tombolos which extend from the center of each breakwater in a landward direction (**Figure 3.11**).

During the monitoring year (May 2019 to May 2020), the shoreline showed net losses of roughly -6,800 cy above the MHWL and -5,400 cy above -16 ft-NGVD. During this period the berm advanced by an average of +5.2 ft while the MHWL receded by an average of 7.0 ft. Over this years period of monitoring, two sand bypass operations placed unverified quantities of sand along the Row Boat Row shoreline.

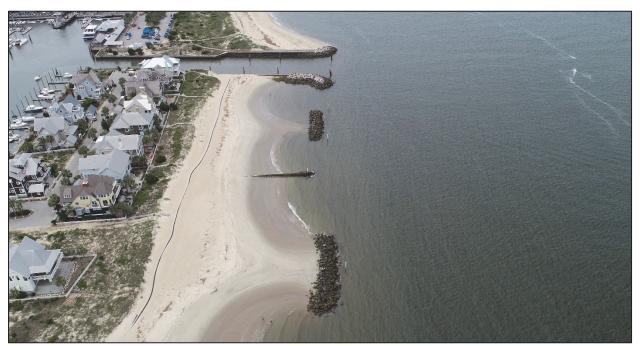


Figure 3.11: Southward looking view of the Row-Boat-Row shoreline detached breakwaters (June 2021 photo).

		Volume Ch	Sh	oreline C	Change (FT)	
	Deceb	Above	A h area	п		MHWL
Station	Reach (ET)	MHWL	Above		erm	
Station	(FT)	(+2.51 FT)	-16 FT	(+)	6 FT)	(+2.51 FT)
-018+72					-27.4	-0.5
	400	-300	-400			
-014+72					-1.2	-0.2
	272	-100	+100			
-012+00					+18.7	+17.8
	400	+700	+1,500			
-008+00					+8.2	+12.5
	400	+800	+1,300			
-004+00					-4.7	+5.5
	100	+100	+100			
Marina						
Total	1,572	+1,200	+2,600	-1.3	(AVG)	+7.0 (AVG)

Table 3.10: Row Boat Row shoreline and volume changes between May 2020 and November 2020.

Table 3.11: Row Boat Row shoreline and volume changes between November 2020 and May 2021.

		Volume Change (CY)		Shoreline C	Change (FT)
		Above			
	Reach	MHWL	Above	Berm	MHWL
Station	(FT)	(+2.51 FT)	-16 FT	(+6 FT)	(+2.51 FT)
-018+72				+0.7	-18.8
	400	+500	-1,000		
-014+72				+6.4	+13.0
	272	+2,600	+3,000		
-012+00				+31.2	+30.2
	400	+2,900	+3,100		
-008+00				-4.4	-14.2
	400	-400	-1,800		
-004+00				-1.5	-10.0
	100	0	-500		
Marina					
Total	1,572	+5,600	+2,800	+6.5 (AVG)	0.0 (AVG)

Note – Volumes of sand associates with multiple sand bypass operations since 2017 have not been accounted for in these tables.

4.0 JAY BIRD SHOALS BORROW SITE MONITORING (SURVEY) RESULTS

Pursuant to permit requirements for the 2009/10 project, the Jay Bird Shoal borrow site has been surveyed for purposes of monitoring its recovery. Approximately 1.8 Mcy of material was excavated during the 2009/10 project and 1.1 Mcy during the 2018/19 project.

Figure 4.1 depicts the most recent borrow site (May 2021) seabed elevations. This plot represents conditions approximately 2 years post-2018/19 project and 11 years post-2009/10 project conditions. In the plot, the full permitted borrow area limits are shown. The permitted limits are further subdivided into three subareas. For the 2009/10 project, only portions of Area 1 and Area 3 were excavated. For the 2018/19 project, only portions of Area 2 and Area 3 were excavated. Also plotted in the figure are the locations of two dredging exclusion zones¹² (both located in Area 3) and a 200 ft tide gage buffer zone (Area 1 & 2). No excavation was conducted within either the exclusion or buffer zones during the 2009/10 and 2018/19 projects.

Figure 4.2 depicts the seabed elevation change during the Year 2 monitoring period (May 2020 to May 2021). **Figure 4.3** depicts the seabed elevation changes during the 11 years (134 months) since 09/10 project construction to the most recent survey (March 2010 to May 2021).

Table 4.1 summarizes the volume changes within the permitted borrow site limits between the monitoring surveys conducted since constriction of the 2009/10 project. During the Year 11 monitoring period (May 2020 to May 2021), the second year following the 2018/19 project excavation, the entire permitted borrow site gained +77,300 cy (inclusive of the exclusion and buffer zones). Within just the latest excavated areas (Areas 2 & 3), the borrow site gained +73,600 cy. Within just these areas, the average seabed elevation increased from -19.2 ft-NGVD to -18.9 ft-NGVD over this period.

Table 4.2 summarizes the volume of material theoretically remaining above the permitted cut elevation (-22 ft-NGVD) by survey date and subarea. These volumes are exclusive of the exclusion and buffer zones shown in **Figure 4.1**. As of May 2021, there are approximately +1,230,700 cy of material available within the permitted borrow site limits.

¹² By permit, no work was allowed within 150 feet and 100 feet of two potential shipwreck sites located within the Jay Bird Shoals borrow site.

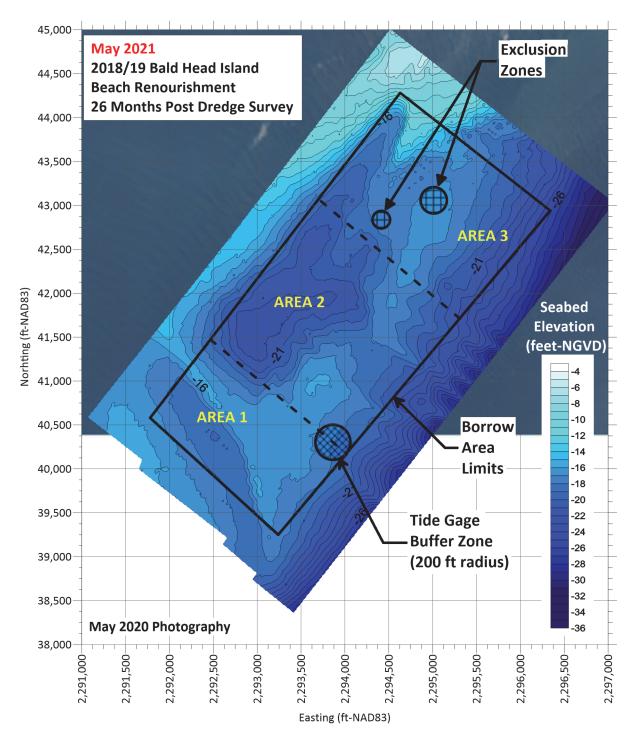


Figure 4.1: Jay Bird Shoals borrow site conditions in May 2021 (26 months post-dredge 2018/19 project).

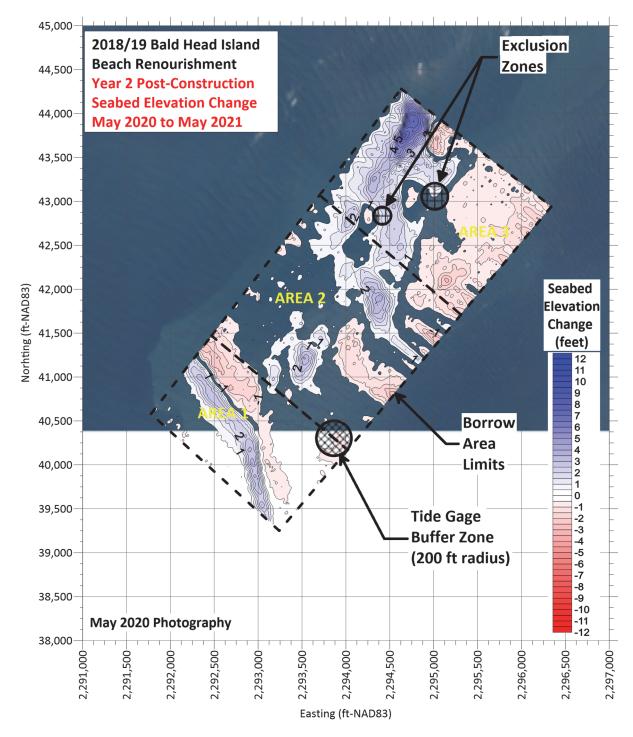


Figure 4.2: Jay Bird Shoals seabed elevation changes during the Year 2 Post-Construction for the 2018/19 project (May 2020 to May 2021).

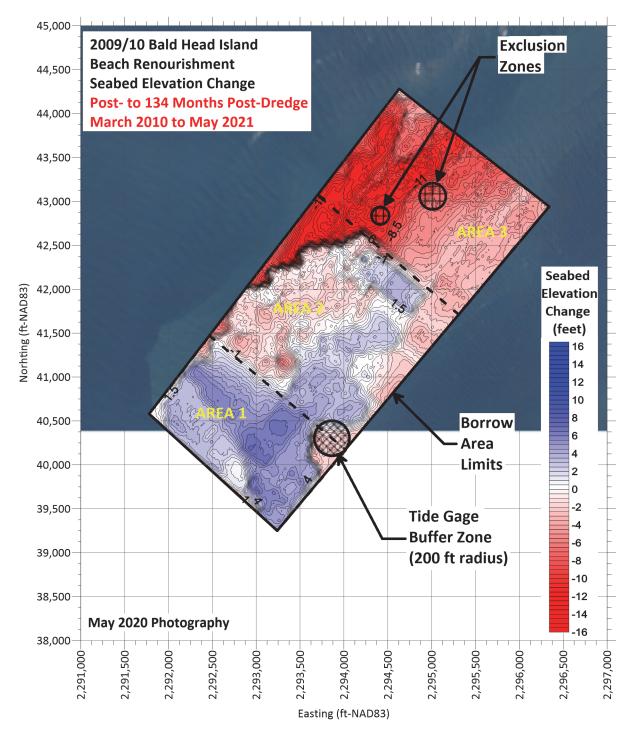


Figure 4.3: Jay Bird Shoals seabed elevation changes since the initial 2009/10 project completion (March 2010 to May 2021).

		Volume Change (CY)				
Survey Period	Duration	Gross Gain	Gross Loss	Net Change		
		Galli	LUSS	Net Change		
October 2009 to March 2010 (Construction)	5 months	+52,700	-1,888,400	-1,835,700		
March 2010 to May 2011 (Year 1 Post-Construction)	14 months	+307,200	-104,800	+202,400		
May 2011 to May 2012 (Year 2 Post-Construction)	12 months	+112,700	-107,200	+5,500		
May 2012 to May 2013 (Year 3 Post-Construction)	12 months	+178,700	-77,600	+101,100		
May 2013 to April 2015 (Years 4 & 5 Post-Construction)	23 months	+286,000	-217,100	+68,900		
April 2015 to May 2017 (Years 6 & 7 Post-Construction)	25 months	+144,900	-328,500	-183,600		
May 2017 to May 2018 (Year 8 Post-Construction)	12 months	+136,800	-71,400	+64,400		
May 2018 to September 2018	4 months	+24,400	-246,300	-221,900		
September 2018 to December 2018	3 months	+188,700	-5,400	+183,300		
December 2018 to March 2019 (BD/AD 18/19 Project)	3 months	+63,700	-1,229,300	-1,165,600		
March 2019 to May 2020 (Year 1 Post-2018/19)	14 months	+239,200	-105,600	+133,600		
May 2020 to May 2021 (Year 2 Post-2018/19)	12 months	+199,000	-121,800	+77,300		
Since 2009/10 Construction (March 2010 to May 2021)	136 months	+1,881,300	-2,615,000	-734,600		
Since 2018/19 Construction (March 2019 to May 2021)	26 months	+438,200	-227,400	+210,900		

 Table 4.1: Jay Bird Shoals borrow site volume changes (<u>PERMITTED</u> LIMITS).

	Volume above -24 ft-NGVD (CY)				
Survey	Area 1	Area 2	Area 3	Total	
October 2009 (Pre-2009/10 Excavation)	812,200	1,593,100	1,330,000	3,735,300	
March 2010 (Post-2009/10 Excavation)	89,100	540,900	1,291,600	1,921,600	
May 2011 (1 Year Post-2009/10)	157,900	685,600	1,275,500	2,119,000	
May 2012 (2 Years Post-2009/10)	154,900	734,400	1,237,900	2,127,200	
May 2013 (3 Years Post-2009/10)	186,300	844,000	1,200,200	2,230,500	
April 2015 (5 Years Post-2009/10)	232,300	992,800	1,081,500	2,306,600	
May 2017 (7 Years Post-2009/10)	289,300	942,100	898,800	2,130,200	
November 2017 (7.5 Years Post-2009/10)	297,400	969,600	923,000	2,190,000	
May 2018 (8 Years Post-2009/10)	315,200	966,400	912,800	2,194,400	
September 2018 (8.5 Years Post-2009/10)	318,600	862,600	800,000	1,981,200	
December 2018 (Pre-2018/19 Excavation)	355,000	945,200	858,900	2,159,100	
March 2019 (Post-2018/19 Excavation)	398,700	332,900	286,200	1,017,800	
May 2020 (1 Year Post-2018/19)	383,000	356,600	410,900	1,150,500	
May 2021 (2 Year Post-2018/19)	387,700	379,000	464,000	1,230,700	

Table 4.2: Jay Bird Shoals borrow site theoretical volume available above -22 ft-NGVD.

Notes:

(1) The red shaded areas for the pre-/post 2009/10 and 2018/19 surveys are the areas excavated during those projects.

(2) Material in thin layers is not accessible to an ocean-certified dredge.

5.0 ONGOING PLANNED OR PROPOSED ACTIVITIES

5.1 Development of a Frying Pan Shoals Borrow Site

In early 2017, the Village submitted permit applications with associated in-depth geotechnical studies and environmental analyses necessary to develop a long-term (and large scale) borrow site located within Frying Pan Shoals. The purpose of such a borrow site was to ensure compliance with Permit conditions necessitating the maintenance of the updrift fillet associated with the 2015 terminal groin project and to allow for large-scale beach renourishment of South Beach. Historically, sand placement from an alternate site has been required due to the scheduled hiatus in the disposal of channel maintenance sand on Bald Head Island by the Wilmington District, USACOE. To that end, pursuant to the exiting tenets of the Wilmington Sand Management Plan, all beach quality channel maintenance material excavated in the summer of 2018 was placed at Oak Island.

In June 2017, the National Marine Fisheries Service (NMFS) issued concerns related to permits associated with the near-term use of the Frying Pan Shoals (FPS) borrow site *without first exploring and exhausting other viable sand source alternatives*. Realistically, the only alternate borrow area available for near-term sand placement at Bald Head Island (BHI) was sand remaining in the previously permitted Jay Bird Shoals (JBS) borrow site. Accordingly, in consideration of the NMFS request, the Village agreed to withdraw their application and prioritize the use of the previously authorized borrow site permitted at JBS (including both a partially "recovered" area dredged in 2009/10 and the remaining undredged portion of the borrow site). With the virtual depletion the Jay Bird Shoals borrow site, resulting from the 2018/19 renourishment project, the Village has reinitiated the permitting of a long-term borrow site located within Frying Pan Shoals in 2019. At the request of the Wilmington District, USACOE, Regulatory Branch, the permit request has been submitted as a modification of the 2015 Terminal Groin permit. As of July 2021, the permit application continues to be in the RAI stage.

5.2 Wilmington Harbor Navigation Project

In early 2021, the Wilmington District, USACOE constructed to perform a routine navigation channel maintenance operation for the Smith Island Range as well as Bald Head Reaches 1 and 2. All beach compatible material excavated was placed on South Beach, Bald Head Island. The measured volume placed was about 1.61 Mcy, mol. At the time of disposal, the sand tube groin field was again buried in its entirety below the beach disposal project berm.

5.3 Wilmington Harbor Deepening Project

In 2019, the Port of Wilmington, NC both sponsored and formulated a Section 203 Report which proposes a plan to deepen and widen (in places), the Federal navigation project, which extends from the Atlantic Ocean up the Cape Fear River to the Port of Wilmington. The Village of Bald Head Island has formally submitted comments to the record which address deficiencies in the project analyses and which requests clarification to impacts addressed or unaddressed by the consultant prepared report. No responses or additional information were received by the Village over the last 12 month period.

6.0 SUMMARY AND CONCLUSIONS

The most recent Wilmington Harbor Inner Ocean Bar maintenance dredging of Bald Head Shoal Channel Ranges 1 and 2, and the Smith Island Channel range was performed in the months of January - April 2021. Federal surveys show approximately 1.6 Mcy of sand during that operation were placed along South Beach pursuant to the terms of the Wilmington Harbor Sand Management Plan (WHSMP). Bald Head Island will likewise be the recipient of the next *future* beach disposal operations in accordance with the continued implementation of the present day WHSMP. The last sand placement project constructed by the Village was between 13 January 2019 and 22 March 2019. The borrow site for that project was Jay Bird Shoals. The final fill volume (in-place) was 1.1 Mcy which included the addition of a Post-Florence FEMA Claim for documented storm related losses from the *engineered beach* in September 2018 (Olsen 2018). The limits of that fill extended eastward only to Sta. 146+00.

As part of the assessment for the 2019 beach renourishment project constructed at South Beach by the Village, it was determined that numerous sand tube groins had reached the end of their effective life and that replacement was warranted. Permits allowed for both an extension of time beyond April 1st for *both removal and replacement* of all remaining thirteen (13) sand tube groins (and underlayments). The work was initiated on/about 13 February and Substantially Completed by 22 March 2019. A *Post-Construction Report*, formulated to document the 2018-2019 project, details all elements of work performed by both contractors (Olsen 2019).

By about 2013, the results of a comprehensive annual beach monitoring program initiated in 2000 by the Village of Bald Head Island yielded the conclusion that sand placement alone could *not* successfully offset navigation channel impacts to the west end of South Beach which had been typically manifest in chronic rates of erosion and a consistent northerly recession of the shorefront. Accordingly, the Village was ultimately forced to "change the existing dynamic" by constructing a single terminal groin designed to complement the placement of future beach fills at the persistent South Beach erosional "hot spot". The project was permitted to be constructed in two phases – with Phase 2 being optional. Simplistically, the structure was designed to serve as a "template" for fill material placed eastward thereof on South Beach. The Phase 1 1,300 ft. long terminal groin (completed in Nov. 2015), was designed however as a "leaky" structure (*i.e.* semi-permeable) so as to provide for some level of continued sand transport to West Beach and portions of the Point (located both westward and northward of the groin stem). Through May 2021, terminal groin project performance – based upon post-construction monitoring – has been both as intended – and as predicted.

Between November 2000 and April 2021, Bald Head Island has received about 8.6 Mcy, mol of sand from the initial widening/deepening and five (5) subsequent maintenance dredging operations for the Wilmington Harbor Navigation Project entrance channel. Including the 2019

project, the Village has placed another 3.2Mcy of sand along the West Beach and South Beach shorelines. Accordingly, in the net Bald Head Island has experienced a total estimated sand placement volume of approximately 11.8 Mcy since 2000 at those two locations – with South Beach receiving 97% or more of the total.

Conversely, the gross volumetric sediment loss over a November 2000 to November 2020 (pre-disposal) monitoring timeframe is conservatively computed at 8.036 Mcy, or approximately 401,800 cy per year – on "average". This annualized "loss" addresses the continuous section of Bald Head Island shorefront extending from the marina entrance to the Cape Fear spit. The assignment of an average annual long-term rate of sand loss at Bald Head Island however, has not necessarily been a meaningful indicator of navigation project impact. Such an average rate is often temporally biased by periods of beach fill equilibration, groinfield "effectiveness due to reconstruction," recent storm events (such as Hurricanes Florence, Dorian and Isaias), the occurrence of episodic destabilization dredging events in close proximity to the island, as well as other physiographic phenomena temporally affecting annualized quantities of alongshore sediment transport – from Bald Head Island. In addition, the island's littoral system continues to adjust to the quasi-stabilizing effect of the terminal groin in existence only since 2015. Along South Beach per se, there has existed historically a "nodal point" some 7,000 ft. eastward of the terminal groin (approx. STA 116+00). At or close to the nodal point, the directionality of *net littoral transport* on an annual basis changes from West (toward the groin) to East (toward Cape Fear). Note – depending on wave climatology, the condition and exposure of the sand tube groinfield, as well as other factors, the effective location of the nodal point can vary slightly along South Beach from year to year. Currently, within the 22,755 shoreline influenced by sand episodically placed since 2000, up to 3.75 Mcy remain in the littoral system (measured above elevation -16 ft. NGVD 29). This *includes* the 1.6 Mcy beach disposal project recently completed in early April, 2021 by the Wilmington District, USACE.

Although not directly impacted by long-term navigation channel improvements and maintenance of the Cape Fear River entrance, the Village Council elected to initiate monitoring of the East Beach shorefront at Bald Head Island beginning in November 2008. Since that time, it is documented that East Beach can undergo strong seasonal variations of beach width and profile volume to a large degree dependent upon storm frequency and intensity, as well as the ever-changing configuration of the Cape Fear spit. For example, the most recent May 2021 survey data show a negligible net shoreline volumetric change of approximately 400 cy (above elevation -16 ft NGVD). throughout the 6,000 ft East Beach shoreline lying northward of Cape Fear over the last 12 months. In the prior year, it had eroded by almost 89,000 cy – due to storm impacts. Between November 2008 and May 2021, the total change had been +350,100 cy. Most of the volume increase had been associated with recent post-storm accretion of the Cape Fear spit shoreline facing Onslow Bay.

Typically, periods of episodic accretional configurations of the Cape Fear spit deemed beneficial to East Beach have corresponded to a high rate of erosion and duneline recession along the easternmost section of South Beach – directly seaward and westward of the Shoals Club facility. For example, between 2000 and 2020, the average MHWL erosion rate at that general location has been over -20 ft/yr – due to sand losses either directly or indirectly associated with the configuration of the Cape Fear spit formation. The most recent (2021) federal disposal project placed fill within 2,000 ft. mol. of the Shoals Club and Cape Fear. This sand source may serve to reduce the most recent erosional cycle evident at the easternmost of South Beach.

In 2021, the Village performed monitoring of the Jay Bird Shoals borrow site utilized to construct the non-federal 1.85 Mcy beach fill sponsored by the Village in 2009/10 and the 1.10 Mcy beach constructed in 2018/19. During the Year 11 monitoring period (May 2020 to May 2021), the second year following the 2018/19 project excavation, the entire permitted borrow site gained 77,300 cy (inclusive of the exclusion and buffer zones). As of May 2021, there is approximately 1.23 Mcy of material located within the permitted borrow site limits above the permitted cut elevation (-22 ft-NGVD). Much of that material is *not* however practically available for dredging at this time.

After an extension of the two marina entrance channel jetties in 2015, temporarily reduced shoaling within the navigation channel resulted in a corresponding reduced volume of disposal sand being place along the Row Boat Row shoreline. Although the Village had planned to continue to proactively bypass sand from the south jetty fillet (located at the distal end of West Beach) to the Row Boat Row shorefront, it became clear that the existing four (4) low level timber groins were not capable of providing an acceptable level of shoreline stabilization at that location.

Hence, near the end of the 2017 monitoring period, the Village initiated construction of two (2) shore parallel detached rock breakwaters located north of the marina entrance seaward of the Row Boat Row shoreline. The placement of breakwaters between existing groins northward of the marina entrance was intended to combine the attributes of each of the two types of stabilization structure so as to reduce the rate of sediment transport from the eroding shoreline caused principally by ferry/barge generated waves. The subject expanded shore stabilization project (detached breakwaters *and* existing groinfield) was designed to have a sand fill prior to construction. The source of the fill was the exiting Bald Head Creek borrow area. A previously permitted Bald Head Creek borrow area was dredged in early 2017 by Marcol Dredging. Some 26,000 cy were placed at Row Boat Row prior to breakwater implementation. Since that time multiple channel maintenance/sand bypass operations have occurred – most with increasing volumes dredged. Typically, dredging is required twice a year on average. This is primarily due to an increasing northerly rate of sediment transport along West Beach caused by a continuing reconfiguration of the Point. As a result, the Village has been forced to perform an increased frequency of bypassing of sand farther northward of the stabilizing influence of the breakwaters.

This required a 2020 modification of the permits associated with the limits of allowable beach disposal seaward of Row Boat Row.

In the spring of 2019, the Village resubmitted permit applications accompanied by indepth geotechnical studies and environmental analyses intended to develop a long term (and large scale) supplementary borrow site located within Frying Pan Shoals. The purpose of such a borrow site would be to both ensure compliance with Permit conditions necessitating the maintenance of the updrift fillet associated with the 2015 terminal groin project and to provide an interim source of beach quality material sufficient to meet future South Beach renourishment requirements – when pursuant to the existing tenets of the Wilmington Harbor Sand Management Plan, beach quality channel maintenance material excavated would be placed at Oak Island. Permitting for this borrow site continues.

An important secondary precept of the spring of 2019 beach fill project constructed by the Village was to allow for the replacement of a sand tube groinfield which had become damaged over time. During the spring 2021 federal channel maintenance project, the groin field in its entirety was again covered by beach fill. That disposal project completed in early April 2021 placed 1.61 Mcy of sand between Sta.60+00 and Sta.121+00, mol. on South Bend.

The original Permits for construction of the terminal groin at Bald Head Island stipulated that if the permittee elected to dredge more than 250,000 cy from the Jay Bird Shoals borrow site after 2015, limited monitoring of the eastern end of Oak Island must be performed. Accordingly, in November 2018, the Village initiated the requisite monitoring at Oak Island (Caswell Beach). The first report of findings for Oak Island followed a November 2019 monitoring survey. A second year monitoring report was issued in December 2020. In early 2021 it was formally agreed by DCM and the USACOE that based upon the results of the Year 2 report, the *Village's responsibility for continued monitoring of Oak Island has terminated*.

In 2019, the Port of Wilmington, NC both sponsored and formulated a Section 203 Report which proposes a plan to deepen and widen (in places), the Federal navigation project, which extends from the Atlantic Ocean up the Cape Fear River to the Port of Wilmington. The Village of Bald Head Island formally submitted several series of comments to-the-record which addressed deficiencies in the project analyses and which requested clarification to impacts addressed, or unaddressed by the consultant prepared report.

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